Appendix H

Groundwater Anti-degradation Analysis

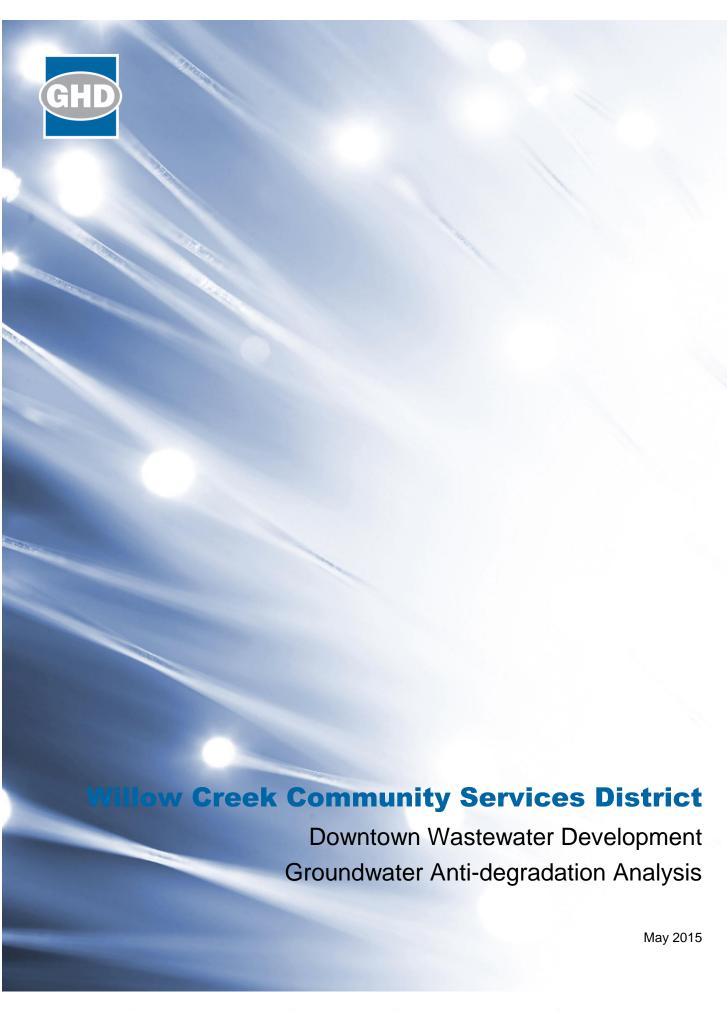


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Appendices

Appendix A – Soils Investigation

1. Introduction

1.1 Project Overview

The unincorporated community of Willow Creek, CA (see Figure 1: Location map) does not currently have a community-wide wastewater collection system, and all of the residences and businesses rely solely on individual septic systems to provide for their wastewater needs. Many of these septic systems are decades old, and some are beginning to fail, impacting ground and surface water in the area and negatively affecting public health and the environment.

To improve public health and increase the opportunity for economic development in the downtown Willow Creek area, the Willow Creek Community Services District (WCCSD or District) is proposing to construct a new wastewater collection, treatment, and disposal system. The proposed project includes a gravity collection system of wastewater in the downtown Willow Creek area, conveyance to a recirculating gravel filter (RGF) treatment plant, and disposal through a subsurface leach field at the Mill Site (see Figure 2: Collection, treatment, and disposal system site plan). The proposed project service area will consist of an estimated 103 total connections to the wastewater system (57 commercial and 46 residential).

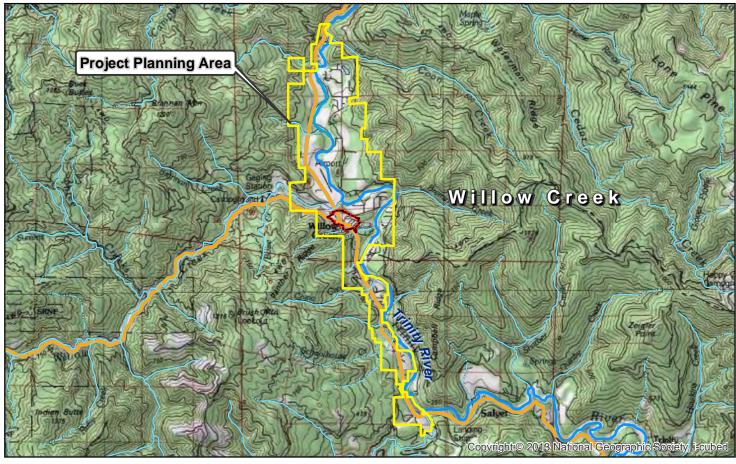
1.1.1 Recirculating Gravel Filter System

A RGF system is a non-proprietary system that uses a community septic tank for primary treatment. After the initial settling of solids, the pre-treated wastewater flows into a recirculation tank. It is pumped from there and intermittently applied uniformly to pea gravel filters. This alternately rests and loads the gravel media, resulting in the development of a microbiological film. As the wastewater percolates down through the gravel filter, it comes into contact with this film, which oxidizes the waste by using it as a food source. It also gets aerated which nitrifies the wastewater. The filtered wastewater is collected at the bottom by an impermeable liner, and the filtrate is then piped back to the recirculation tank and mixed with the effluent from the septic tank. A portion of the flow is split back to the beginning of the septic tank where it is mixed with the raw sewage for denitrification. The recirculation process is designed so that on average, the wastewater flows through the media and then is retained in the recirculation tank for 5 cycles. This alternates the flow from being aerobic to anoxic/anaerobic, which results in very good rates of BOD, TSS, ammonia, and nitrate removal. When the recirculation tank becomes full, a control valve closes and the returning flow from the filter is discharged as effluent. A general flow diagram of this process is illustrated in Figure 3.

This process has been implemented in the community of Weott, and the community of Miranda also has a similar facility that uses sand as the media instead of pea gravel. Both of these systems are of similar size to the proposed Willow Creek system and have produced excellent quality effluent for over 25 years (Miranda was constructed in 1980 and Weott in 1990.)







Paper Size 8.5" x 11" (ANSI A) 1.5 2 Miles
Map Projection: Mercator Auxiliary Sphere
Horizontal Datum: WGS 1984
Grid: WGS 1984 Web Mercator Auxiliary Sphere

California State Highway

U.S. Highway





Willow Creek Community Services District Boundary

Potential Service Area

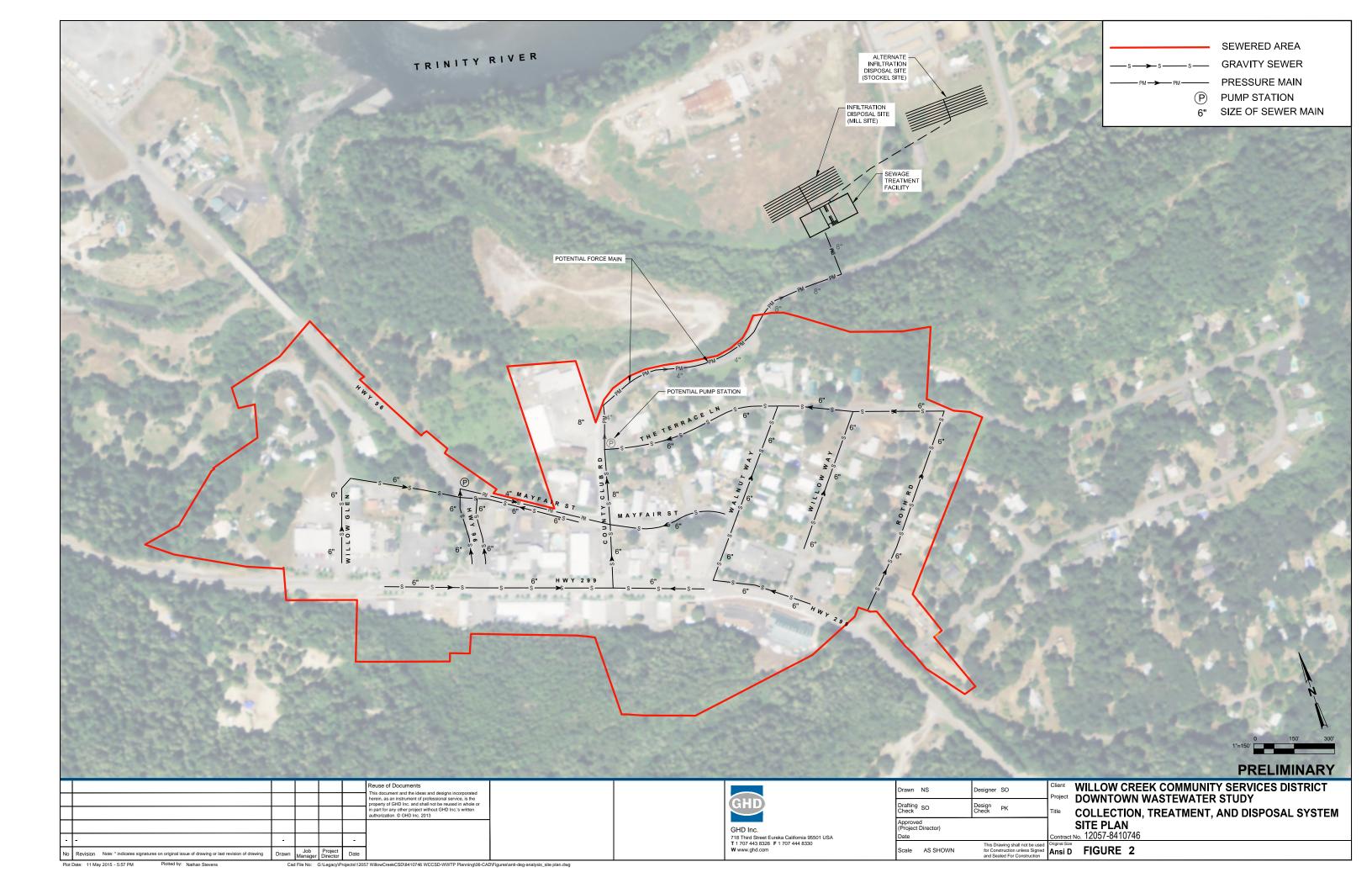
Willow Creek Community Services District

Perennial Stream

Intermittent Stream Trinity River

> Job Number | 8410746.01 14 Oct 2014

Location Map



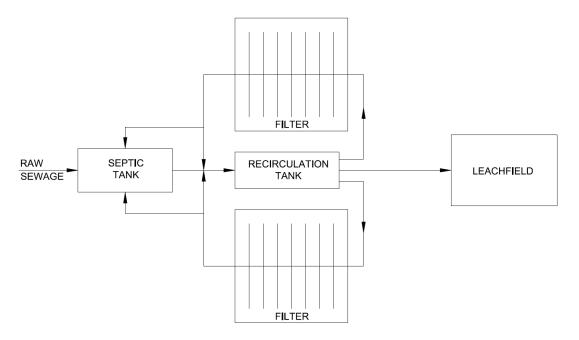


Figure 3: General flow diagram of recirculating gravel filter system

1.2 Purpose of Analysis

This *Groundwater Anti-degradation Analysis* has been prepared for the WCCSD Downtown Wastewater Development Project to ensure that the proposed leach field disposal at the Mill Site or the alternate disposal site, the Stockel Site, will not result in degradation to the beneficial uses of groundwater at the site or the beneficial uses of the nearby Trinity River. All of the effluent from the wastewater treatment plant will be disposed of via a subsurface leachfield. Because of this, nitrate is the key constituent of concern, and other constituents (e.g. BOD, TSS, etc.) are not considered in this analysis. The analysis compares water quality objectives for nitrate with the projected nitrate concentrations in the groundwater upon mixing with infiltrated treated effluent. The degradation determination is made by comparing the projected commingled concentration of the groundwater with the most stringent water quality objectives.

This analysis will also be used to confirm that substantial groundwater mounding will not occur, and that an acceptable groundwater clearance beneath the system will be maintained.

1.3 Beneficial Uses

Protection of water quality is based on maintenance of beneficial uses. Section 13050(f) of California's Porter-Cologne Water Quality Control Act states the following:

"Beneficial uses" of the waters of the state that may be protected against quality degradation include, but are not limited to, domestic, municipal, agricultural and industrial supply; power generation; recreation; aesthetic enjoyment; navigation; and preservation and enhancement of fish, wildlife, and other aquatic resources or preserves."

The 2011 Water Quality Control Plan for the North Coast Region (Basin Plan) lists the following as existing (E) or potential (P) beneficial uses of groundwater:

- Municipal and Domestic Supply (E)
- Agricultural Supply (E)

- Industrial Service Supply (E)
- Industrial Process Supply (P)
- Native American Culture (E)
- Aquaculture (P)

The Basin Plan also lists the following as existing or potential beneficial uses of the Willow Creek Hydrologic Subarea of the Trinity River:

- Municipal and Domestic Supply (E)
- Agricultural Supply (E)
- Industrial Service Supply (E)
- Industrial Process Supply (P)
- Groundwater Recharge (E)
- Freshwater Replenishment (E)
- Navigation (E)
- Hydropower Generation (E)
- Water Contact Recreation (E)
- Non-Contact Water Recreation (E)
- Commercial and Sport Fishing (E)
- Cold Freshwater Habitat (E)
- Wildlife Habitat (E)
- Rare, Threatened, or Endangered Species (E)
- Migration of Aquatic Organisms (E)
- Spawning, Reproduction, and/or Early Development (E)
- Shellfish Harvesting (P)
- Aquaculture (P)

These existing and potential beneficial uses are each taken into consideration when assessing the potential for degradation to groundwater and surface water resources.

2. Background Data

There are several factors to consider when determining the potential for groundwater degradation and mounding. Pertinent data has been gathered to characterize the soils and the groundwater at the Mill Site and Stockel Site. Where data has not been gathered (e.g. for treatment plant effluent flows), assumptions have been made to arrive at reasonable conclusions.

2.1 Soils Investigation and Groundwater Monitoring

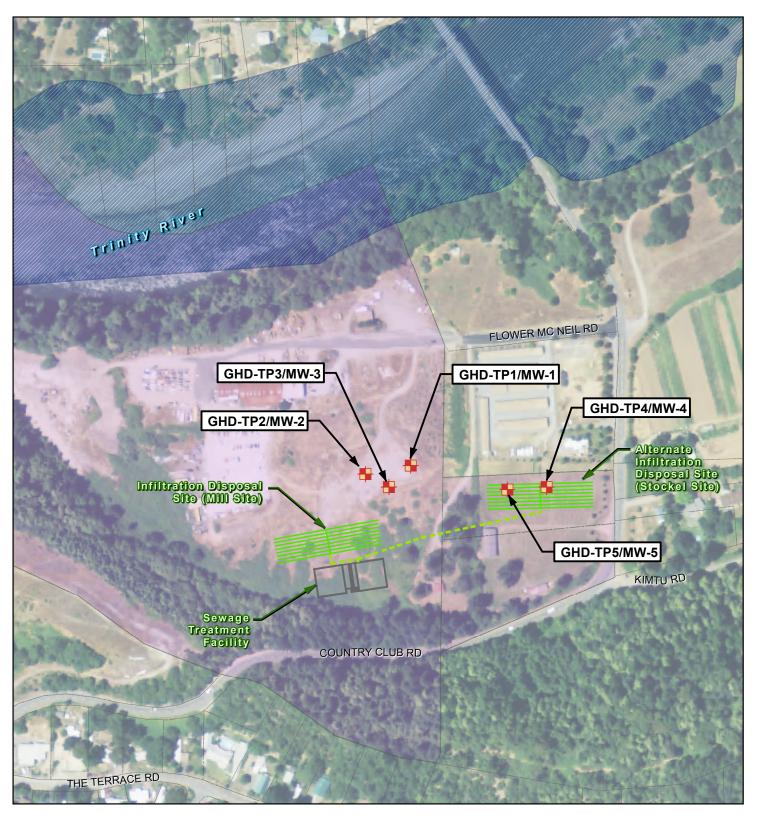
In 2014, GHD investigated the soils and groundwater at the Mill Site and Stockel Site (see Appendix A for the report, including soil boring logs, lab results, and depth to groundwater measurement data). Three test pits (GHD-TP1 through GHD-TP3) were constructed at the Mill Site (Figure 4), and depths ranged from approximately 13 feet below ground surface (bgs) to 16.5 ft bgs (where groundwater was encountered). Soils encountered were generally alluvial deposits, and they ranged from sand to sandy loam. All of the Mill Site samples fell into Zone 1 of the USDA Soil Percolation Suitability Chart.

Two test pits (GHD-TP4 and GHD-TP5) were constructed at the Stockel Site (Figure 4) at depths of 12 ft and 13 ft bgs, respectively. Groundwater was not encountered in either of these test pits. Soils encountered at this site were generally silty loam. The sample gathered from GHD-TP4 fell into Zone 2 of the USDA Soil Percolation Suitability Chart, and the sample from GHD-TP5 fell into Zone 1.

Three monitoring wells (MW-1 through MW-3) were installed in the test pits at the Mill Site (Figure 4). Depth to groundwater measurements were collected on a weekly basis for approximately 11 weeks, from the end of March through mid-June 2014. Approximately 0.5 inches of rainfall was recorded during this time, and depth to groundwater measurements generally varied from approximately 11 feet bgs to 13 feet bgs. Two monitoring wells (MW-4 and MW-5) were also installed in the test pits at the Stockel Site (Figure 4). These monitoring wells were dry for every depth to groundwater measurement.

2.2 Effluent Flows

As WCCSD does not currently have a community wastewater system, there is no data pertaining to effluent flows. However, GHD developed preliminary design flows in our 2015 Preliminary Engineering Report (PER) titled *Willow Creek Community Services District Downtown Wastewater Development, Preliminary Engineering Report – Wastewater Facilities.* In this PER, GHD developed an estimated average dry weather flow (ADWF) for this system of 38,000 gallons per day. This estimate includes projections for growth and expansion of the community.



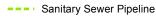


Test Pit/Monitoring Well



Parcel Boundaries

Treatment Facility



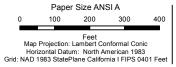
Disposal Sites



FEMA 100 Year Floodplain



Approximation of the 520 foot Elevation for Non-FEMA Mapped Portion of the Trinity River







Willow Creek Community Services District Willow Creek Downtown Wastewater Development

Job Number 8410746.01 A 12 May 2015 Revision Date

Test Pit and Monitoring Well Locations

Figure 4

G:112057 WillowCreekCSD\8410746 WCCSD-WWTP Planning\08-GIS\Maps\Figures\DischargePermit\F4_TP_MW_Locations.mxd Third Street Eureka CA 95501 USA T707 443 8326 F 707 444 8330 E eureka@ghd.com

3. Projected Nitrate Concentrations

The Basin Plan states that effluent disposal "shall not cause the groundwater nitrate concentration to exceed 10.0 mg/L as N at any source of drinking water on the property nor on any off-site potential drinking water source." Because groundwater is a potential drinking water source, the Regional Water Board will require that the nitrate concentration of the treatment plant effluent be at or below 10.0 mg/L by the time it reaches groundwater. It should be noted that all of the properties at and surrounding the potential effluent disposal sites are on the WCCSD public water system, and there are currently no groundwater wells in the immediate vicinity. It is also unlikely that groundwater wells would be installed downgradient of the sites in the future given the industrial nature of these sites, the lack of dependable available groundwater, and the availability of the public water system.

3.1 Nitrate Loading Analysis

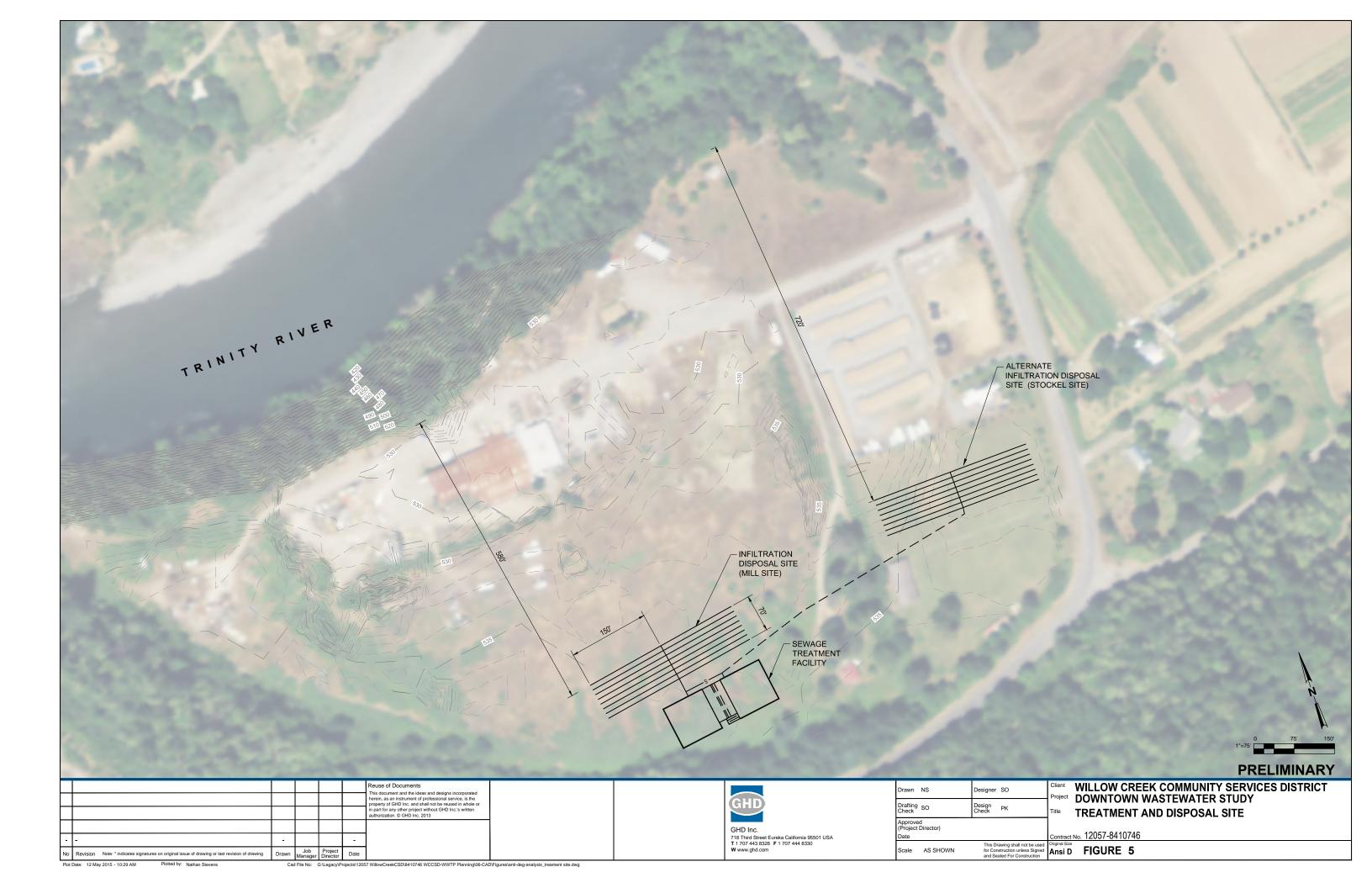
The analysis of nitrate loading effects was based on an estimate of an annual chemical-water mass balance. The assumptions and equations given in this section were used to project what the nitrate concentration will be in the groundwater immediately adjacent to the site.

3.1.1 Groundwater Gradient

While the groundwater gradient for the area has not been quantitatively characterized, it is reasonable to assume, based upon site topography, that groundwater in the area is flowing toward the Trinity River, in a northerly direction (see Figure 5: Treatment and disposal site).

3.1.2 Wastewater Loading

As discussed in Section 2.2, the estimated ADWF for this system is 38,000 gpd. Ramlit (1982) states that an average flow, rather than anticipated peak flows, should be used for this type of mass balance calculation (see Section 6 for reference). The PER discusses a preliminary leach trench design of 18 inches wide and 6 feet deep with 1 foot of cover over the leach pipe. This trench design results in 11.5 square feet of exposed trench area per linear foot of trench. Using this number in conjunction with a design clean water loading rate of 1.5 gpd/ft² (discussed in the PER) and the estimated ADWF of 38,000 gpd, approximately 2,200 feet of leach field trench would be required. This distance of trench can be achieved by digging eight parallel trenches, 10 feet on center, that are each 300 feet long. To facilitate the highest achievable dilution, the proposed leach field will be oriented so that the leach lines are generally perpendicular to the groundwater gradient (parallel to the Trinity River). The width of the disposal field normal to the groundwater gradient would then be 300 feet.



3.1.3 Nitrate Loading

The community of Weott, CA has a wastewater treatment plant that is very similar in size and design to what is proposed for WCCSD. Data from the Weott plant was used as a basis for predicting what the effluent nitrate concentration will be at the WCCSD plant. The effluent from the Weott treatment plant has been measured at 13 mg/L, so the assumption was made that the effluent from the WCCSD plant will have this same nitrate concentration of 13 mg/L on average.

It should be noted that the design of the Weott system did not include returning a portion of the filter effluent back into the septic tank, which the WCCSD design will include. Given that this will increase denitrification, it is reasonable to assume that the nitrate concentrations in the Willow Creek system effluent will generally be lower than 13 mg/L.

3.1.4 Denitrification Losses in the Soil Column

Ramlit (1982) states that nitrate reduction by means of soil denitrification is typically within the range of 15 to 25%. A denitrification ratio of 25% was used for this analysis based on favorable soil conditions.

To account for denitrification losses in the soil column, the nitrate concentration of wastewater percolate draining to groundwater can be calculated as follows (Ramlit Associates, 1982):

$$N_n = N(1 - DN)$$
 Eqn. 1

where:

- N_p is the wastewater percolate nitrate concentration (mg/L)
- N is the assumed effluent nitrate concentration (13 mg/L)
- DN is the denitrification value (assumed to be 0.25)

Using the values mentioned above, the wastewater percolate nitrate concentration is calculated to be 9.75 mg/L at the point where the percolate reaches groundwater.

4. Groundwater Mounding

The Basin Plan states that a "groundwater mounding analysis shall be used to predict the highest rise of the water table and shall account for background groundwater conditions during the wet weather season." It also mentions that a minimum groundwater clearance of 24 inches shall be maintained beneath an infiltration system such as what is proposed for this project. With the preliminary 6-foot-deep trench design and the shallowest observed depth to groundwater being 11 feet, along with minimal potential for groundwater mounding, issues with meeting this minimum clearance are not anticipated.

The potential for significant groundwater mounding at the site is anticipated to be very low, as the soil samples taken from this site were all classified as sand, and treatment plant effluent will percolate quickly. The total leach field area has been preliminarily sized to be 300 feet by 70 feet (21,000 square feet). An average dry weather flow of 38,000 gpd (5,080 cubic feet per day) and an assumed soil porosity of 0.4 would result in 7.3 inches per day of percolate over the leach field area. The measured long-term percolation rate at the site was 30 gpd/ft² (see Appendix A), which correlates to approximately 10.0 feet of percolation per day. Because the long-term percolation rate is much higher than the amount of percolate that will be generated, significant groundwater mounding is not anticipated.

5. Conclusion

To improve public health and increase the opportunity for economic development in the downtown Willow Creek area, the Willow Creek Community Services District (WCCSD or District) is proposing to construct a new wastewater collection, treatment, and disposal system. The proposed project includes a gravity collection system of wastewater in the downtown Willow Creek area, conveyance to a recirculating gravel filter (RGF) treatment plant, and disposal through a subsurface leach field

Using previous information that has been gathered at the Weott treatment plant (similar system to the proposed in design and scale), nitrate levels are anticipated to be at or below 13 mg/L coming out of the treatment plant. Further nitrate reduction occurs naturally in the soil column. By the time the wastewater percolate reaches groundwater, it can be assumed that 25% denitrification will occur, and the nitrate concentration in the percolate at this point will be less than 10.0 mg/L.

Furthermore, considering the expected leach field loading rate and the natural long-term percolation rate at the site, significant groundwater mounding is not anticipated. This analysis has documented that the proposed treatment and disposal system will not impact groundwater or the Trinity River in a manner that will be in violation with the Basin Plan.

6. Works Cited

- GHD Inc. (2015). Downtown Wastewater Development, Preliminary Engineering Report Wastewater Facilities. Eureka, CA.
- North Coast Regional Water Quality Control Board. (2011). Water Quality Control Plan for the North Coast Region. Santa Rosa, CA.
- Ramlit Associates. (1982). Assessment of Cumulative Impacts of Individual Waste Treatment and Disposal Systems. Berkeley, CA.



Appendix A – Soils Investigation

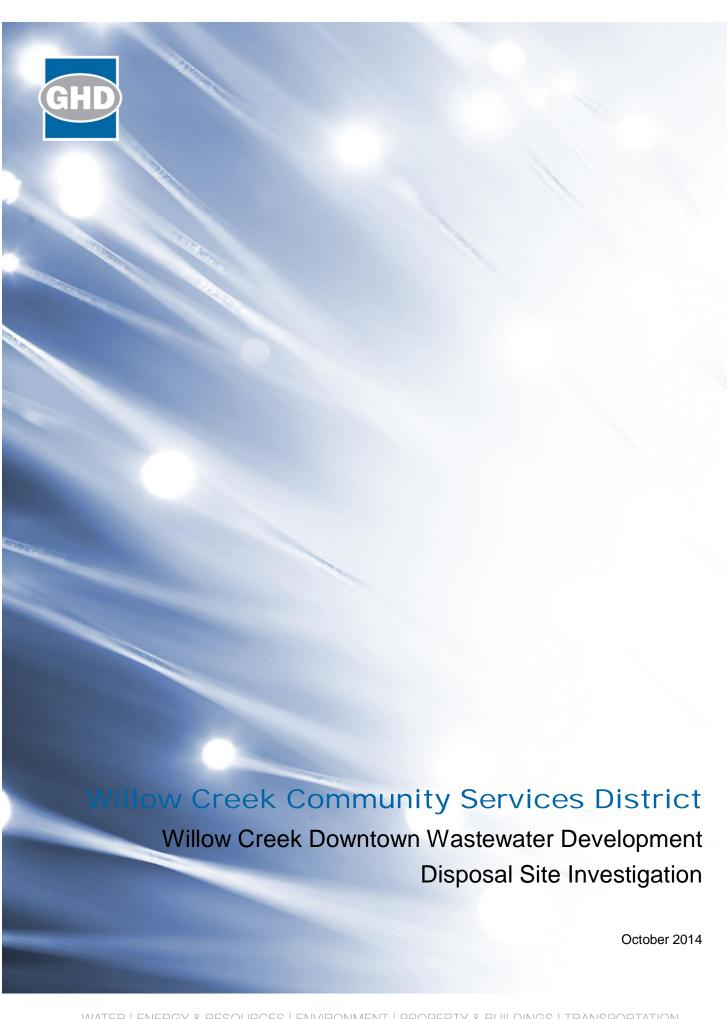


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Appendices

Appendix A – Trinity Valley Consulting Engineers Veterans Memorial Park Soils Report

Appendix B – GHD Test Pit Logs

Appendix C – Materials Testing Data Sheets

Appendix D – Groundwater Monitoring Well Log

1. Introduction

The Willow Creek Community Services District (Willow Creek CSD) evaluated four potential disposal locations for a community wide wastewater disposal system. The project planning area is shown on Figure 1. Potential disposal site locations are shown on Figure 2 and include areas within Veterans Memorial Park, the former mill site, Stockel, and Trinity Valley Elementary School properties. An evaluation of soils located at Veterans Memorial Park was completed by Trinity Valley Consulting Engineers (TVCE). A copy of the TVCE soil report for Veterans Memorial Park is included in Appendix A. GHD oversaw the completion of test pits at three of the potential wastewater disposal sites; the former mill site (Figure 3), Stockel (Figure 3), and Trinity Valley Elementary School (Figure 4) properties.

2. Soil Investigation

2.1 Soil Investigation Methodology

In order to characterize soil type and collect samples for laboratory analysis, eight test pits (GHD-TP1 through GHD-TP8) were completed using either a backhoe or excavator at the three potential disposal sites evaluated by GHD. The test pits were completed to varying depths from approximately 12 to 18 feet below ground surface (bgs). Soils were logged from the ground surface to approximately four feet bgs from the sidewalls of the test pit. Soil excavated from depths greater than four feet bgs were characterized through observation and samples obtained from the backhoe or excavator bucket. Soils were logged following the United States Department of Agriculture (USDA) Soil Survey Manual (Soil Survey Division Staff, 1993). Soil color was noted using Munsell Soil-Color Charts. Logs for the test pits completed at the former mill site (GHD-TP1 through GHD-TP3), Stockel (GHD-TP4 and GHD-TP5), and Trinity Valley Elementary School (GHD-TP6 through GHD-TP8) properties are included in Appendix B.

2.2 Potential Disposal Site Soils

Generally, soils encountered at each of the potential disposal locations are alluvial deposits either within the floodplain (Trinity Valley Elementary School property) or on the uplifted, raised terraces above the Trinity River. Soils at the three disposal sites varied from silt loam observed at the Stockel property to sand and sandy loam at the former mill site, to gravelly sands at the Trinity Valley Elementary School property. Generally, soils were finer (silts and clays) at the Stockel property and increased in grain size with proximity to and elevation relative to the Trinity River.

The test pits completed at the Trinity Valley Elementary School property (GHD-TP6 through GHD-TP8) were generally coarser and included cobbles up to approximately 18 inches in diameter from approximately 8 feet bgs to the completed depths of 14 feet bgs, 16.5 feet bgs, and 18 feet bgs. The location of the potential disposal system at the Trinity Valley Elementary School is approximately 1,000 feet from the Trinity River and is not located within the Federal Emergency Management Agency (FEMA) 100-year flood zone. However, anecdotal evidence of community members and staff at the Willow Creek CSD indicates that this location was inundated in the 1964 flood (500-year flood event). The former mill site and Stockel properties are located approximately 40 and 50 feet, respectively, above the Trinity River.

Groundwater was encountered at the former mill site at approximately 13 feet bgs in test pits GHD-TP1 and GHD-TP3 and 16.5 feet bgs in test pit GHD-TP2. Groundwater was not encountered in the excavations completed on the Stockel and Trinity Valley Elementary School properties.

2.3 Material Testing

Representative samples of soil encountered in the test pit were collected and retained for grain size analysis. GHD completed grain size analysis via the American Society for Testing and Materials (ASTM) Standard Test Method for Particle-Size Analysis of Soils (ASTM D422) for nine soil samples. Grain size percentages were plotted on the USDA Soil Percolation Suitability Chart. The results of grain size analysis are presented in Table A (below). Copies of the materials testing data sheets are included in Appendix C.

Table A - Potential Disposal Sites Soil Classification and Percolation Suitability
Zone

Potential Disposal Site	Sample Location	Sample Depth (feet bgs)	Soil Classification and Soil Percolation Suitability Chart Zone
Former Mill site	GHD-TP1	5 to 5.5	Sand-Zone 1
Former Mill site	GHD-TP2	5 to 5.5	Sand- Zone 1
Former Mill Site	GHD-TP3	8 to 8.5	Sand- Zone 1
Stockel Property	GHD-TP4	7 to 7.5	Loamy Sand- Zone 2
Stockel Property	GHD-TP5	5 to 5.5	Sand- Zone 1
Trinity Valley Elementary School Property	GHD-TP6	3 to 3.5	Sand- Zone 1
Trinity Valley Elementary School Property	GHD-TP7	4 to 4.5	Sand- Zone 1
Trinity Valley Elementary School Property	GHD-TP7	16 to 16.5	Sand- Zone 1
Trinity Valley Elementary School Property	GHD-TP8	4 to 4.5	Loamy Sand- Zone 2

2.4 Monitoring Well Construction

Seven monitoring wells were installed in test pits completed at the former mill site, Stockel, and Trinity Valley Elementary School properties. Three monitoring wells, GHD-MW-1 through GHD-MW-3 were completed at the former mill site property. Two monitoring wells (at each property) were completed in the test pits excavated at the Stockel (GHD-MW-4 and GHD-MW-5) and Trinity Valley Elementary School (GHD-MW-6 and GHD-MW-7) properties.

The monitoring wells were constructed using two inch diameter blank and factory slotted (0.010 inch) poly vinyl chloride pipe (PVC) which was placed along the sidewall and the test pit was backfilled with the excavated soils. The top of the casing for each well was completed above the ground surface (ags) to facilitate depth to water measurements, locating, and provide visibility for vegetation maintenance (grass cutting) at each of the properties. Well construction details are included on the test pit logs (Appendix B) and presented in Table B (on the following page).

Table B - Monitoring Well Construction- Former Mill Site, Stockel, and Trinity Valley Elementary School Properties

Monitoring Well	Total Depth (feet bgs)	Screen Interval (feet bgs)	Top of Casing Elevation (inches ags)
GHD-MW-1	13	8 to 13	30
GHD-MW-2	16.5	11.5 to 16.5	29
GHD-MW-3	14	10 to 14	14
GHD-MW-4	12	7 to 12	38
GHD-MW-5	13	8 to 13	21
GHD-MW-6	14	4 to 14	32
GHD-MW-7	16.5	6.5 to 15.5	31

2.5 Groundwater Monitoring

Depth to groundwater measurements were collected on a weekly basis for approximately eleven weeks from the time of well construction in monitoring wells GHD-MW-1 through GHD-MW-5. Approximately 0.5 inches of rainfall was recorded during the time period of depth to water measurement collection. Depth to water measurements in monitoring wells GHD-MW-6 and GHD-MW-7 were collected on a weekly basis for three weeks from the time of well construction Depth to water measurements collected from the monitoring wells are presented in Table 1, included in Appendix D.

Generally, groundwater was not observed in monitoring wells at the Stockel and Trinity Valley Elementary School properties. Groundwater was encountered at the former mill site property at depths varying from approximately 11 feet bgs to 13 feet bgs.

3. Percolation Test

A long term percolation test was completed at Veterans Park, the last week of May, 2014, to determine the clean water loading rates and develop an estimate of the hydraulic gradient for the site. The location of the percolation pit is shown on Figure 5.

A backhoe was used to dig an approximately 56 inch long, 20 inch wide, and 28 inch deep trench. The depth of trench was chosen such that the bottom of the trench was at the most restrictive soil layer.

A wooden frame was constructed, on to which a float valve was attached. The frame was smaller in dimension than the trench so that an air gap existed between the frame and the trench side walls. The float valve was then attached to a hose which was connected to a District water value. The float valve was orientated to spray water downward, and to prevent scour or disturbance to the soil at the bottom of the trench, the wooden frame including a horizontal flat member below the water flow to dissipate the spray.

The water was turned on, and the trench filled approximate 14 inches deep with water until the float valve shut the flow off. At this time, the meter was read and reordered. Each day the meter was again read to determine the amount of water that was percolated in the trench each day.

The test was run for seven days. The average flow per day was determined to be 675 gallons per day. Given the total exposed trench area, including the sides and the bottom was 22.6 square foot; the clean water loading rate was determined to be 30 gallons per day per square foot.





Before restoring the site, a backhoe was used to dig a trench perpendicular to the test pit, extending about 8 feet each side of the test pit and about 4 feet deeper that the percolation test pit. During excavation of this trench, a water line was severed and GHD staff had to immediately evacuate the trench as the rate of water flow was extreme and it was impossible to safely measure the depth to saturated soil. GHD's initial measurement of the slope of the wetted interface was at a depth of approximately 11 inches bgs at a distance of approximately two feet from the eastern edge of the percolation test pit. Despite the accuracy of the measurement, GHD is using the slope of the wetted surface as an assumption to the value of hydraulic gradient (i) in Darcy's Law equation (Q=CiA).

4. Conclusions

Based on the data collected from the installation of test pits, grain size analysis of potential disposal site soils, depth to water measurements, and evaluation of the percolation test, GHD has the following conclusions:

- Soils at the three disposal sites varied from silt loam observed at the Stockel property to sand and sandy loam at the former mill site, to gravelly sands at the Trinity Valley Elementary School property.
- Material testing of selected soil samples indicates that soils at the potential disposal sites consist of Sand (Zone 1) and Loamy Sand (Zone 2).
- All of these sites have soils that are suitable for use as disposal site leach fields.

5. Recommendations

Based on the soils investigation completed at the three potential disposal sites, it appears that the soils and location of the former Mill Site property is most suitable for the potential Willow Creek CSD Disposal Site. The soils at the adjacent Stockel site are also suitable. GHD recommends moving forward with the former Mill Site as the future disposal location for the Willow Creek CSD wastewater project. The Stockel site could be used if issues develop with land acquisition of the former Mill Site property.

6. References

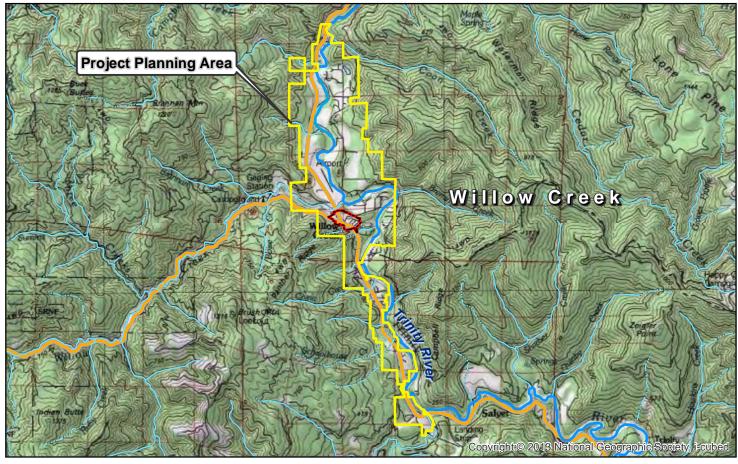
Soil Survey Division Staff, Soil Survey Manual. Soil Conservation Service, United States Department of Agriculture, Handbook 18, 1993.

American Society for Testing and Materials, Standard Test Method for Particle-Size Analysis of Soils ASTM D422-63(2007).

Figures







Paper Size 8.5" x 11" (ANSI A) 1.5 2 Miles
Map Projection: Mercator Auxiliary Sphere
Horizontal Datum: WGS 1984
Grid: WGS 1984 Web Mercator Auxiliary Sphere

California State Highway

U.S. Highway





Willow Creek Community Services District Boundary

Potential Service Area

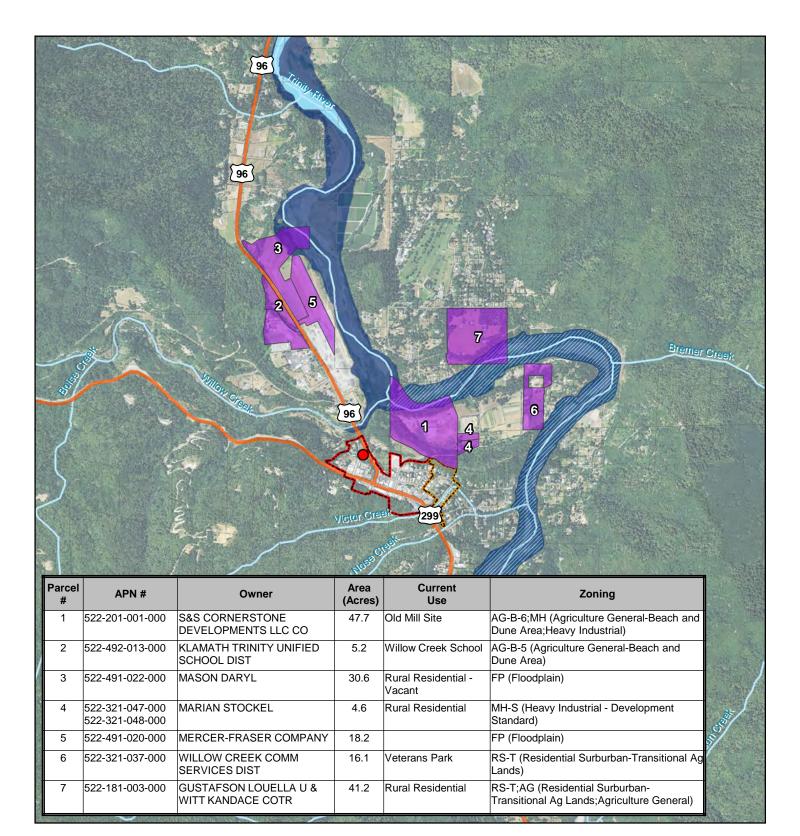
Willow Creek Community Services District

Perennial Stream

Intermittent Stream Trinity River

> Job Number | 8410746.01 14 Oct 2014

Project Planning Area Map





Potential_Disposal_Sites...



Willow Creek Community Services District HQ

Proposed Service Area



FEMA 100 Year Floodplain



Core Expanded

Approximation of the 520 foot Elevation for Non-FEMA Mapped Portion of the Trinity River Highway River

Paper Size ANSI A 500 1,000 1,500 2,000

Feet
Map Projection: Lambert Conformal Conic
Horizontal Datum: North American 1983
Grid: NAD 1983 StatePlane California I FIPS 0401 Feet



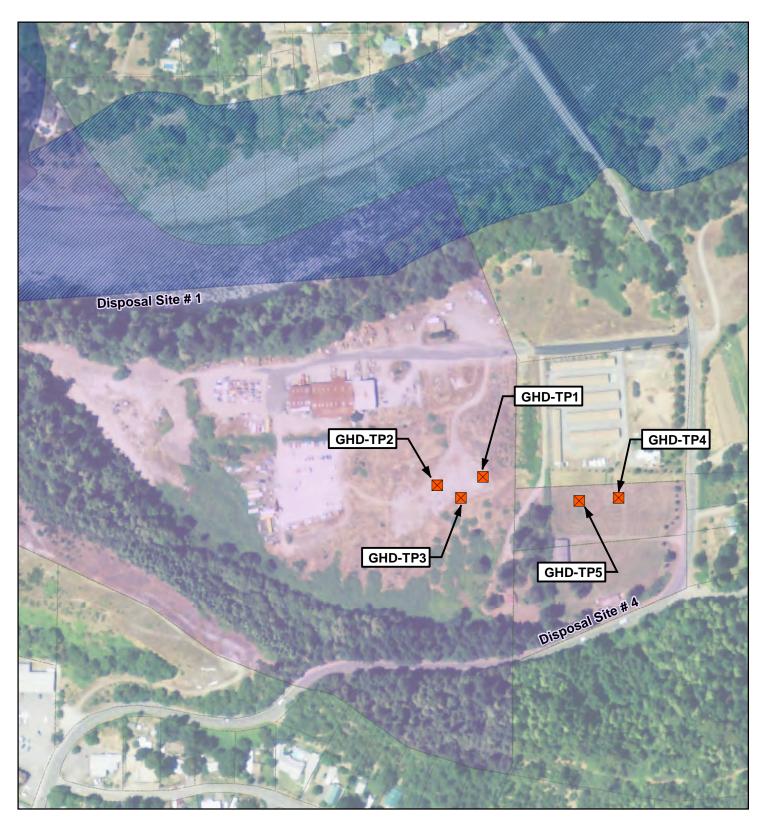


Willow Creek Community Services District Willow Creek Downtown Wastewater Development

Job Number 8410746 Revision

14 Oct 2014 Date

Potential Disposal Sites





Test Pit



FEMA 100 Year Floodplain

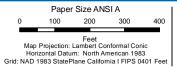


Parcel Boundaries

Potential Disposal Site



Approximation of the 520 foot Elevation for Non-FEMA Mapped Portion of the Trinity River







Willow Creek Community Services District Willow Creek Downtown Wastewater Development

Job Number 8410746.01 Revision 14 Oct 2014 Date

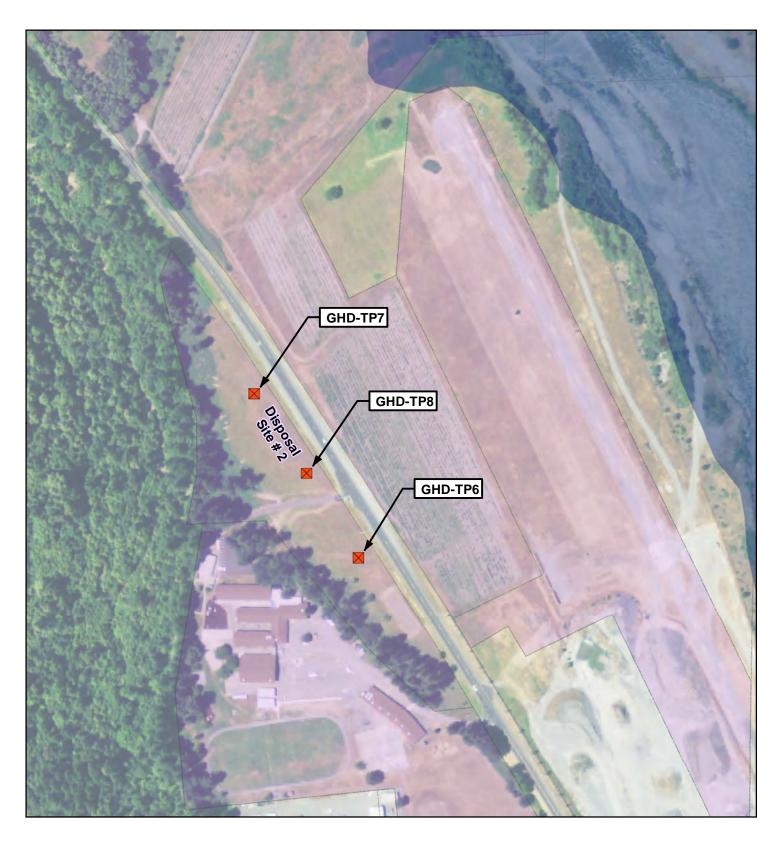
Old Mill Site - Stockel Property

Figure 3

G:\Legacy\Projects\12057 WillowCreekCSD\8410746 WCCSD-WWTP Planning\08-GIS\Maps\Figures\Report X\F3_OldMillSite.mxd Third Street Eureka CA 95501 USA T707 443 8326 F 707 444 8330 E eureka@ghd.com G\Legacy\Projects12057 WillowCreekCSD\8410746 WCCSD-WW IP Planningu8-dismapsin-guresixeport are jurismines.mixa

2013. Whilst every care has been taken to prepare this map, GHD and Willow Creek Community Services District make no representations or warranties about its accuracy, reliability, completeness or suitability for any particular purpose and cannot accept liability and responsibility of any kind (whether in contract, tort or otherwise) for any expenses, losses, damages and/or costs (including indirect or consequential damage) which are or may be incurred by any party as a result of the map being inaccurate, incomplete or unsuitable in any way and for any reason.

Data source: County of Humboldt: Parcels, Data Custodian, Data Set Namer/Title, Version/Date. Created by;irousseau





Test Pit



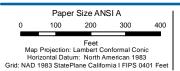
Parcel Boundaries



Potential Disposal Site



FEMA 100 Year Floodplain





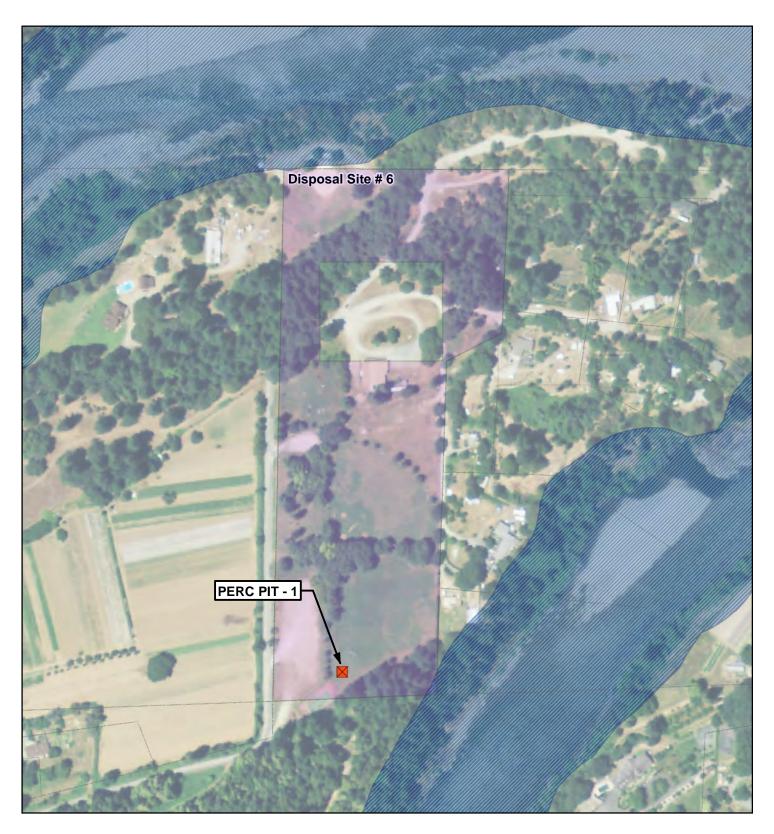


Willow Creek Community Services District Willow Creek Downtown Wastewater Development

Job Number | 8410746.01 Revision Date 20 Oct 2014

Trinity Valley Elementary School

Figure 4





PERC Pit



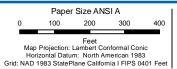
Parcel Boundaries



Potential Disposal Site



FEMA 100 Year Floodplain







Willow Creek Community Services District Willow Creek Downtown Wastewater Development

Job Number | 8410746.01 Revision A 14 Oct 2014 Date

Veterans Park

Figure 5



Appendix A – Trinity Valley Consulting Engineers Veterans Memorial Park Soils Report

ONSITE WASTEWATER INVESTIGATION REPORT

For

VETERANS PARK WILLOW CREEK, CALIFORNIA APN: 522-321-037

Report Provided for:

Willow Creek Community Services District Post Office Box 8 Willow Creek, California 95573

Report Provided By:

Trinity Valley Consulting Engineers, Inc. 67 Walnut Way / Post Office Box 1567 Willow Creek, California 95573 (530) 629-3000 Fax: (530) 629-3011



March 2012 Project Number: 209

Table of Contents

SITE CONDITIONS:	2
PROPOSED PROJECT:	2
SOIL INSPECTIONS:	2

ATTACHMENTS:

- 1. SITE LOCATION MAP
- 2. SOIL PROFILE LOGS
- 3. SOIL TEXTURAL ANALYSIS

Site Conditions:

The proposed project site is located at Veterans Park in Willow Creek, California in the County of Humboldt (attached location map). Latitude and Longitude of the project sites are N 40.9396° and W -123.6174° per Google Earth. The approximate elevation of the property is 521 feet above mean sea level.

The proposed project site is a large grass field currently being used as a soccer field.

Proposed Project:

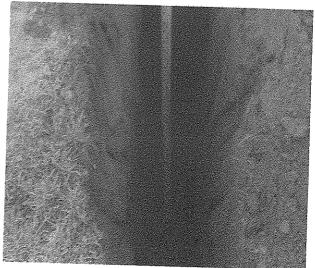
The proposed project for this parcel would be to install a treatment and disposal system for a community wastewater system.

Soil Inspections:

The site soils were investigated on January 12, 2012. Investigation consisted of excavation of three backhoe test pits, soil sampling, and laboratory analysis (see attached layout for test pit locations). Findings were as follows:

Test Pit #1: Soils found in this test pit consisted of a shallow layer of topsoil (less than one foot) followed by a layer of light brown loamy sand to depth of approximately five feet. The bottom five feet of the test pit consisted of a grey sandy gravel (see attached soil log). No groundwater or bedrock was encountered in the investigation. Soil samples were taken at four feet and seven feet, and tested to be Loamy Sand (zone 1), and Sand (zone 1) respectively (see attached test results).







Test Pit #2: Soils found in this test pit consisted of a shallow layer of topsoil (less than one foot) followed by a layer of light brown loamy sand to depth of approximately five feet. The bottom five feet of the test pit consisted of a grey sandy gravel (see attached soil log). No groundwater or bedrock was encountered in the investigation. Soil samples were taken at two feet and six feet, and tested to be Loamy Sand (zone 2), and Sand (zone 1) respectively (see attached test results).







Test Pit #3: Soils found in this test pit consisted of a shallow layer of topsoil (less than one foot) followed by a layer of light brown loamy sand to depth of approximately four feet. The bottom five feet of the test pit consisted of a grey sand with gravels and cobbles (see attached soil log). Excavation was ceased at a depth of eight feet due to trench caving. No groundwater or bedrock was encountered in the investigation. Soil samples were taken at two feet and six feet, and tested to be Sandy Loam (zone 2), and Sand (zone 1) respectively (see attached test results).



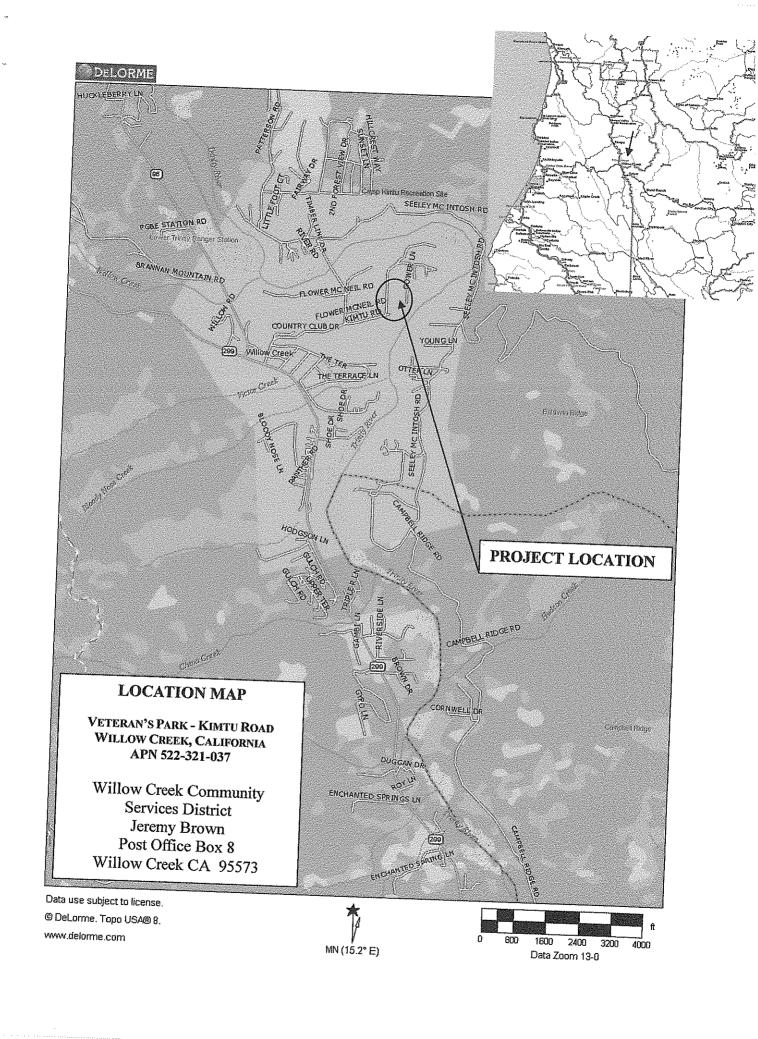




Attachment 1

Site Location Map





Island take to describe the second to

WILLOW CREEK COMMUNITY SERVICES DISTRICT



Attachment 2

Soil Profile Logs



SOIL EXPLORATION LOG

Project Name: WC Wastewater

Project No: 209.10

Date: 1/12/12

APN: 522-321-37

Hole Size: 3'X6'

Excavation Method: Backhoe

Test Hole #: TP-1

Logged by: J. McKnight

DESCRIPTION & REMARKS	COLOR	MOISTURE	CONSIST.	SOIL TYPE - USCS	DEРТН	PROFILE	SAMPLE TYPE / NUMBER	BLOWS / FT	WATER CONTENT %	UNIT DRY WEIGHT PSE
Topsoil w/ Roots & Organics	DK BRN	Damp	Soft							
Loamy Sand	BRN	Moist	Med	SM			****	TP-1A		
Sandy Gravel	Grey	Moist	Med	GS				TP-1B		

SOIL EXPLORATION LOG

Project Name: WC Wastewater

Project No: 209.10

Date: 1/12/12

APN: 522-321-37

Hole Size: 3'X6'

Excavation Method: Backhoe

Test Hole #: TP-2

Logged by: J. McKnight

DESCRIPTION & REMARKS	COLOR	MOISTURE	CONSIST.	SOIL TYPE - USCS	DEРТН	PROFILE	SAMPLE TYPE / NUMBER	BLOWS / FT	WATER CONTENT %	UNIT DRY WEIGHT, PSF
Topsoil w/ Roots & Organics	DK BRN	Damp	Soft		1					
Loamy Sand	BRN	Moist	Med	SM	—-2 —-3 —-4 —			TP-2A		
Sandy Gravel No Bedrock Observed	Grey	Moist	Med	GS	— -6 — -7 — -8 — -9 — -10			TP-2B		

SOIL EXPLORATION LOG

Project Name: WC Wastewater

Project No: 209.10

Date: 1/12/12

APN: 522-321-37

Hole Size: 3'X6'

Excavation Method: Backhoe

Test Hole #: TP-3

Logged by: J. McKnight

DESCRIPTION & REMARKS	COLOR	MOISTURE	CONSIST.	SOIL TYPE - USCS	ОЕРТН	PROFILE	SAMPLE TYPE / NUMBER	BLOWS / FT	WATER CONTENT %	UNIT DRY WEIGHT, PSF
Topsoil w/ Roots & Organics	DK BRN	Damp	Soft		1					
Sandy Loam	BRN	Moist	Med	SM	2 3 4			TP-3A		
Sandy Gravel with rounded cobbles to 6" in diameter	Grey	Moist	Med	G G	5 6 7 8 9 10			TP-3B		

Attachment 3

Soil Textural Analysis



Job No. 209.10 Page 1 of 4

Date: 01/18/2012

Report to:

Willow Creek Community Services District

Post Office Box 8

Willow Creek, CA 95573

RE:

Veteran's Park - Kimtu Road

Willow Creek, California 95573

APN: 522-321-37

Hole #: TP-1A & 1B

Depth: 4' & 7'

Sample Description: Soil

Sampled By: J. McKnight

Date Tested: 01/18/2012

Date Sampled: 01/12/2012

SOILS EXAMINATION FOR SOIL PERCOLATION SUITABILITY

 Textural Analysis
 TP-1A
 TP-1B

 Sand:
 86%
 88%

 Clay:
 02%
 05%

 Silt:
 12%
 07%

 Zone Classification:
 1
 1

Bulk Density: N/A

Comments:

Zone 1 - Soils in this zone are very high in sand content. They readily accept effluent, but because of their low silt and clay content, they provide minimal filtration. These soils demand greater separation distances from ground water.

Zone 2 - Soils in this zone provide adequate percolation rates and filtration to effluent. They are suitable for use of a conventional system without further testing.

Zone 3 - Soils in this zone are expected to provide filtration of effluent, but their ability at a suitable rate is questionable. These soils require wet-weather percolation tests to verify their suitability for effluent disposal by conventional leachfield methods.

Zone 4 - Soils in this zone are unsuitable for a conventional leachfield because of their severe limitations for accepting effluent.

Josh McKnight, P.E.

Soil Texture Analysis Worksheet

Job Name: Willow Creek Community Services District

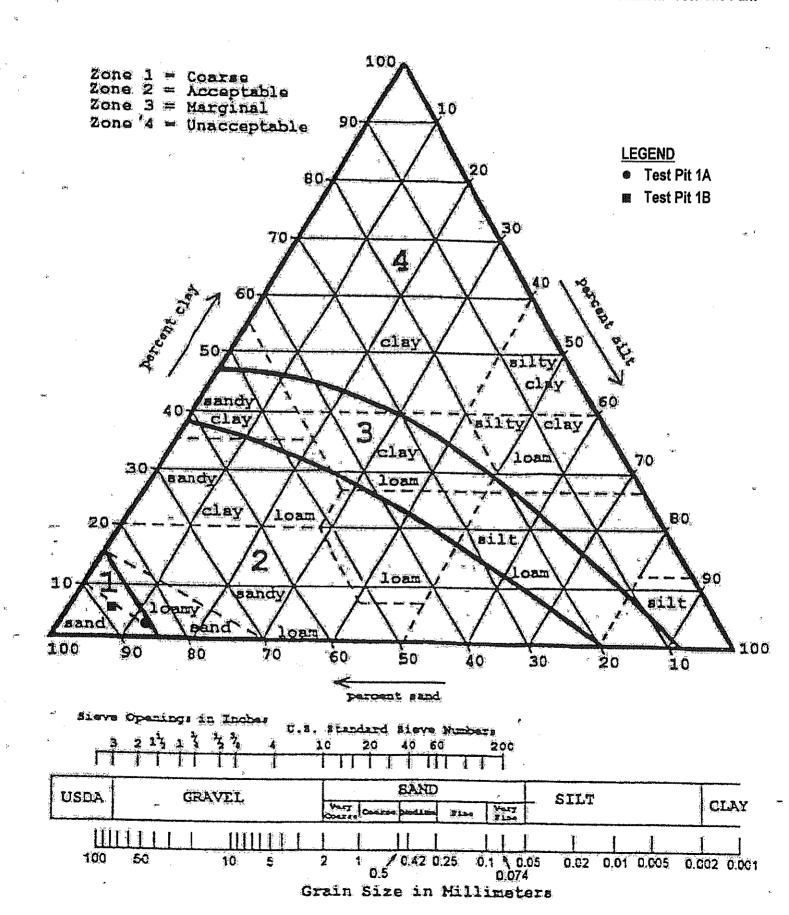
Location: Veterans Park
Performed By: J. McKnight

Hole #	TP-1A	TP-1B
Depth (ft)	4'	7'
Oven Dry Weight (g)	99.9	99.9
Starting Time	1150	1145
Temp @ 40 Sec	65	63
Hydrometer Reading @ 40 sec	21	19
Composite Correction	7.1	7.5
True Density @ 40 sec	13.9	11.5
Temp @ 2 Hours	67	67
Hydrometer Reading @ 2 Hours	08	11
Composite Correction	6.5	6.5
True Density @ 2 hours	1.5	4.5
% Sand	86	88
% Clay	2	5
% Silt	12	7
Soil Zone	1	1
Classification	Loamy Sand	Sand

Job Name: Willow Creek Community Services District Location: Veterans Park

Test Pit Number	TP-1A	TP-1B
Percolation Rate (minutes per inch)	15	6
Application Rate (gallons per day per square foot)	0.8	1.1

Willow Creek Community Services District Location: Veterans Park



Job No. 209.10 Page 1 of 4

Date: 01/18/2012

Report to:

Willow Creek Community Services District

Post Office Box 8

Willow Creek, CA 95573

RE:

Veteran's Park – Kimtu Road Willow Creek, California 95573

APN: 522-321-37

Hole #: TP-2A & 2B

Depth: 2' & 6'

Sample Description: Soil

Sampled By: J. McKnight

Date Tested: 01/18/2012

Date Sampled: 01/12/2012

SOILS EXAMINATION FOR SOIL PERCOLATION SUITABILITY

Textural Analysis	TP-2A	TP-2B
Sand:	80%	91%
Clay:	05%	02%
Silt:	15%	07%
Zone Classification:	2	1

Bulk Density: N/A

Comments:

Zone 1 - Soils in this zone are very high in sand content. They readily accept effluent, but because of their low silt and clay content, they provide minimal filtration. These soils demand greater separation distances from ground water.

Zone 2 - Soils in this zone provide adequate percolation rates and filtration to effluent. They are suitable for use of a conventional system without further testing.

Zone 3 - Soils in this zone are expected to provide filtration of effluent, but their ability at a suitable rate is questionable. These soils require wet-weather percolation tests to verify their suitability for effluent disposal by conventional leachfield methods.

Zone 4 - Soils in this zone are unsuitable for a conventional leachfield because of their severe limitations for accepting effluent.

Josh McKnight, P.E.

Soil Texture Analysis Worksheet

Job Name: Willow Creek Community Services District

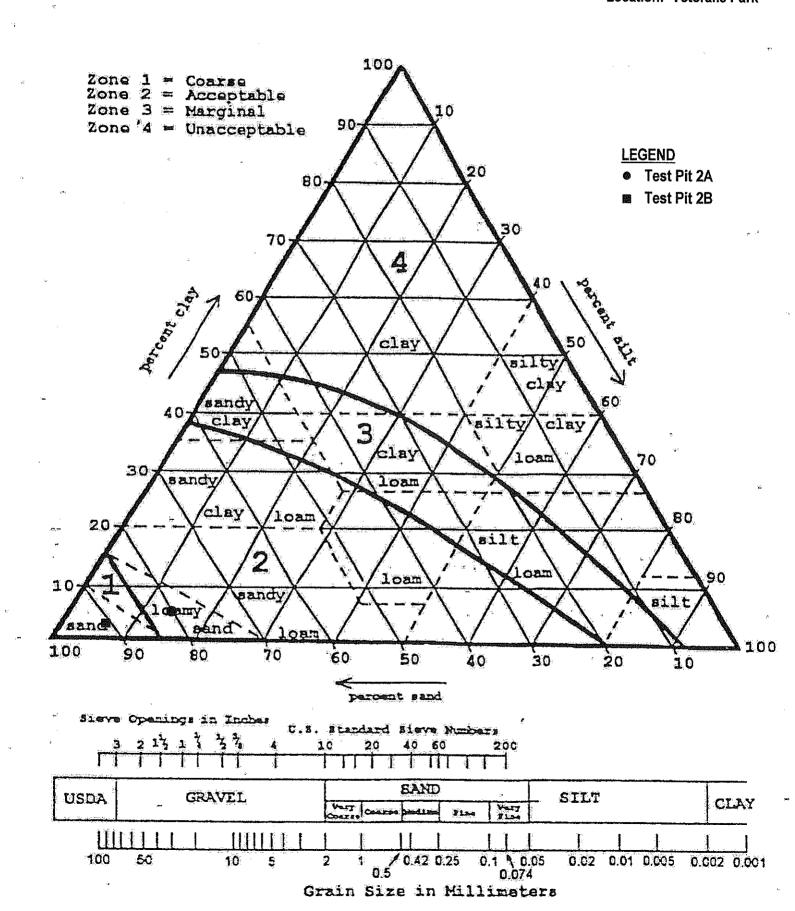
Location: Veterans Park Performed By: J. McKnight

Hole #	TP-2A	TP-2B
Depth (ft)	2'	6'
Oven Dry Weight (g)	99.9	99.9
Starting Time	1255	1245
Temp @ 40 Sec	67	66
Hydrometer Reading @ 40 sec	26	16
Composite Correction	6.5	6.9
True Density @ 40 sec	19.5	9.1
Temp @ 2 Hours	72	71
Hydrometer Reading @ 2 Hours	11	08
Composite Correction	5.7	5.9
True Density @ 2 hours	5.3	2.1
% Sand	80	91
% Clay	5	2
% Silt	15	7
Soil Zone	2	1
Classification	Loamy Sand	Sand

Job Name: Willow Creek Community Services District Location: Veterans Park

Test Pit Number	TP-2A	TP-2B
Percolation Rate (minutes per inch)	15	6
Application Rate (gallons per day per square foot)	0.8	1.1

Willow Creek Community Services District Location: Veterans Park



Job No. 209.10 Page 1 of 4

Date:

01/18/2012

Report to:

Willow Creek Community Services District

Post Office Box 8

Willow Creek, CA 95573

RE:

Veteran's Park - Kimtu Road

Willow Creek, California 95573

APN: 522-321-37

Hole #: TP-3A & 3B

Depth: 2' & 6'

Sample Description: Soil

Sampled By: J. McKnight

Date Tested: 01/18/2012

Date Sampled: 01/12/2012

SOILS EXAMINATION FOR SOIL PERCOLATION SUITABILITY

Textural Analysis TP-3A TP-3B Sand: 69% 91% Clay: 11% 02% Silt: 20% 07%

Zone Classification:

2 1

Bulk Density: N/A

Comments:

Zone 1 - Soils in this zone are very high in sand content. They readily accept effluent, but because of their low silt and clay content, they provide minimal filtration. These soils demand greater separation distances from ground water.

Zone 2 - Soils in this zone provide adequate percolation rates and filtration to effluent. They are suitable for use of a conventional system without further testing.

Zone 3 - Soils in this zone are expected to provide filtration of effluent, but their ability at a suitable rate is questionable. These soils require wet-weather percolation tests to verify their suitability for effluent disposal by conventional leachfield methods.

Zone 4 - Soils in this zone are unsuitable for a conventional leachfield because of their severe limitations for accepting effluent.

Josh McKnight, P.E.

Soil Texture Analysis Worksheet

Job Name: Willow Creek Community Services District

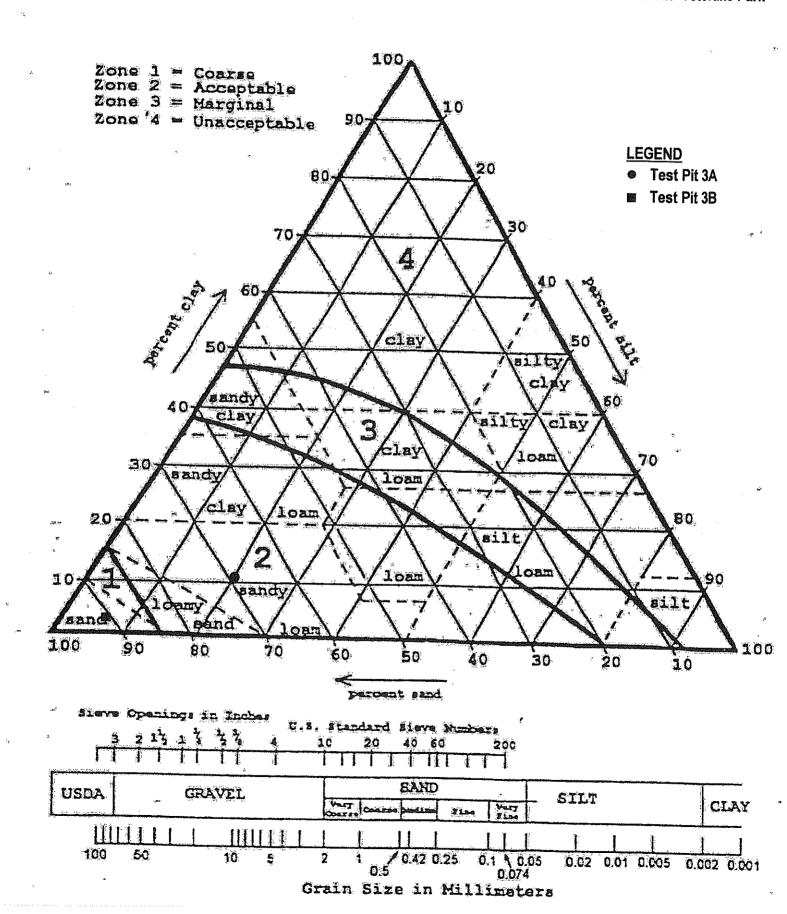
Location: Veterans Park Performed By: J. McKnight

Hole #	TP-3A	TP-3B
Depth (ft)	2'	6'
Oven Dry Weight (g)	99.9	99.9
Starting Time	1140	1250
Temp @ 40 Sec	69	67
Hydrometer Reading @ 40 sec	37	15
Composite Correction	6.3	6.5
True Density @ 40 sec	30,7	8.5
Temp @ 2 Hours	67	71
Hydrometer Reading @ 2 Hours	17	08
Composite Correction	6.5	5.9
True Density @ 2 hours	10.5	2.1
% Sand	69	91
% Clay	11	2
% Silt	20	7
Soil Zone	2	1
Classification	Sandy Loam	Sand

Job Name: Willow Creek Community Services District Location: Veterans Park

Test Pit Number	TP-3A	TP-3B
Percolation Rate (minutes per inch)	16	6
Application Rate (gallons per day per square foot)	0.7	1.1

Willow Creek Community Services District Location: Veterans Park



Appendix B – GHD Test Pit Logs

FIELD SOIL BORING AND WELL LOG GHO-TPI Project No.: 8410746.04 Sheet Project Name: WILLOW CREEK CSD Method of Drill: EXCAVATOR Location: MILLSITE Boring Diameter: TEST PIT = APPROX 4 hat whe Logged By: ANNA GOWER Total Depth of Hole: 13 feet 668 Drilling Co: KEN SMITH Vegetation: BARE GROUNN SPARSE GRASS 25 MARCH 2014 Depth to Groundwater: APPAN MARELY Aspect: 1200 13 FEET BG(Slope: SAMMES AT STEET BUS AND 13 FEET BUS GHD-MW-1 Soil Description Well Comments Color Diagram Moisture Structure Description & Remarks

Description & Remarks	(moist)	Moisture	Structure			TOC: 30"A45
GRAVELLY LOAMY SAND	7.51/4/2	DRY	MASSIVE COMPLETEL		_ 1′ _	
SANDY LOHM	7.5Y	/	MASSIVE COMPACTED		2′	
	3/1	DRY.	COMMENTER		_ 3′ _	7
SANDY LOAM	7.54 4/2	DRY	SINGLE GRA	V - Cam Los	4′ _	RIL FROM
	7.54 412	DRY	SINGLE	4 SAMPLE (X	_ 5′ _	30" AGS TO 8 FATEGS
GRAVEVY SANA					6′ _ _ 7′ _	
	7.5 Y 4/2	DRY	SINGLE	Sublainted GRAVEL (-288	_ / _ _ 8′ _	21
					9′	2"FACTORY SLOTTED ALC STATES ALC
	5/R 4/10	DRY -> MOIST	SINGLE	APPROX. 70%	10′ _	FACT BGS
			GRAN	SUBROUNDED CARAVEL * SAMPLE	11′ _	and the second s
					12′ - 	
BENOCK	Por	TOM OF	EXCAMON.	GROUNDWHER ZX ENCOUNTEREDS AT APPROX, 13 FORT	_	7
	-			BSS	14′ -	ENDCAP WI HOLE !
					15′ -	





		-				Pit ID:GHD-TP2
Project Name: Willow Cra	eok C	SD	Project No.: 8410746-04 Sheet \ of 2			
Method of Drill: excavo			Location:	Mill Site		
Boring Diameter: Pit			Logged By:	LLW		
Drilling Co .: Kens wa	ter ter	dor	Date:	3/25/14		
	5mith		Total Depth		5' b	95
Vegetation: bare grow	nd lare	a55	Depth to Gr		-11	95
Slope/Aspect: $\angle 2^{\circ}/_{\circ}$	10		Soil Map Un	it:		
Samples: 5-5.5' and	8-8.51		Matches Ma	ap Unit 🗌 Yes	□ N	o Maybe
Soil De	scription				ے	GHO-MWZ
Description & Remarks	Color (moist)	Moisture	Structure	Comments	Depth	Well Diagram 100=29095
gravelly	2.543/2	dry	mussive	Sravel 30%		149,0
J loamy sand	T Cage	019	Compaded	Color mixed	 _ 1' _	
J			1	from sorce		0-1151
Sandy lam	104R2/2		11	gravel 5%		211 8440
9 100	10112			wood debris 50%	2′	Solid PVC
		g commence of the commence of			– –	Q D
					_ 3′ _	
sand	543/2		25.8180	gravel 2016		
sand	2.543/3		to1fslk	gravel 20%	_ 4′ _	
Sano	2.01-13			J. 1001 20 16		
250 A		**************************************		\$ Sample - DE	5′	
				42 couple -		
					6′	
50.6 - 6 - 6	545/1			gravel 20%		
Course sand	2/7/			J. 20%	7′ _	
		A				
				No. 10 DE	8′ _	
				*Sample →[= -	
	2			8	9′ _	
				ω.	_ 10′ _	
•					_ 11′ _	
					_	9
					_ 12′ _	11.5-16.5
						2" Ectory
					_ 13' _	Slotled PUE
						20
						- Contraction of the Contraction
					 15′ _	
					13 _	A description of the second



							Pit IDGHD-TP2
Project Name: Willow (reek (75D	Project No.: 8410746-04 Sheet 2 of Z				
Method of Drill:			Location: Mill Site				
Boring Diameter:			Logged By:	ILU)		
Drilling Co.:			Date: 3/25/14				
Driller Name:			Total Depth	of Hole:	1		
Vegetation:			Depth to Gr	oundwate	er:		
Slope/Aspect:			Soil Map Ur	nit:			
Samples:			Matches Ma	ap Unit	Yes	_ N	o Maybe
Soil De	scription					4	
Description & Down J.	Color			Comn	nents	Depth	Well
Description & Remarks	(moist)	Moisture	Structure			۵	Diagram
						17	2" G. bon -
					7	- 16-	SIOHEO DIC =
	Termi	neted	ered as	S' ba	<	-	
	GWe	ncount	ered a	6.51%	a 5	-17-	and a A
					1	_	a rician /
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							Pit IDGHD-TP3
Project Name: Willow C	reek (SD	Project No.:	84107	46-0	14	Sheet of
Method of Drill: excano	Hor		Location:	mill	Site		
Boring Diameter: Di			Logged By:	Llin)		
Drilling Co.: Kensuat	er Tend	los-	Date: 3 25/14				
Driller Name: Kenny	Smith !	owner)	Total Depth	of Hole:	' 14	(,0,	655
Vegetation: Sare Sp	MESE CYT	ass	Depth to Gr	oundwate	r: \3	,01k	095
Slope/Aspect: 421%			Soil Map Un				0
Samples: 6-6.5 and	10-10.5	,	Matches Ma	ap Unit	Yes	N	o 🗌 Maybe
Soil De	scription					4	GHD-MW3
Description 9 Demands	Color	N. 4 - 1 - 1	6.	Comm	nents	Depth	Well Diagram
Description & Remarks	(moist)	Moisture	Structure			Ω	TOC = 14'495
gravelly	2.544/2	drn	massive	20% Gr	avels		3
Sandwicken I can	, ,,	3	to	, ,		 - 1' _	
3 3			to If Sbk			_ + _	
						 _ 2' _	
						_	
			18		-	 - 3' _	
		A. C.				_ 3 _	
		William Christian					
						- 4' _	
					+		
Sandy loam	25/2/2		IFSbK	100.0	(2)	, 5' <u> </u>	
serial jours	2.543/2		17 201	10% gr	avery		
<u> </u>					-(+	_ 6′ _	and a second sec
				A Sam	npled _		
		of the latest and the		N Son	Tuez.	- 7′ _	
2 -2 -014		a de la composição de l			-		
somethy .	20131		TICLE	1501 00	anele	_ 8′ _	
sandy ctay loam	2.59 12		142 br	20105	avels		
		4		5-10%	06060	_ 9′ _	
			11				
			IFSDE	140	1101	_ 10′ _	
			1	Asamp	69-191	_	10.0-14.01
			-			- 11' <i>-</i> -	2" fectory
							Slotted PUC
					_	- 12′ _	
						_	
					V	- 13' _—	
						_	
						- 14′ -	endcap w/t
	termin	ated a	encour	1	1.12/1	000000	hole wrapped A
1	Sround	water	- encour	rocd	N12, Ph	S - 15' _	in secfabric
							0

Project Name: WILLDW CREEK CSD Project No.: 8410744.04 Sheet I of I

Method of Drill: BAZKHOE Location: STOCKEL PROPERTY

Boring Diameter: TOTA AT WATH = CHEET Logged By: ANNA GOWER

Drilling Co: KEN SMITH Total Depth of Hole: 12 FEET BLS

Vegetation: GRASS Date: 25 MARCH ZOLL

Aspect: 42% Depth to Groundwater: NOT ENCOUNTERED

Slope: SAMILES AT 3 FEET AND 7 FEET BGS							
Soil De		_	GHD-MW4				
Description & Remarks	Color (moist)	Moisture	Structure	Comments	Depth	Well Diagram ToC= 38"A43	
SILT LOAM	7.54	DRY	IFSBK_		_ 1′ _		
		10°			2′ _		
	×			MADINE X	_ 3′ _ _ 3′ _	\rightarrow	
					_ 4' _ 	2" BLANK AL FROM	
GRAVELLY SILT LOAM	7.54	DRY	IFSBK	APPROX 15% SUSCAINDED GRAVEL	_ 5′ _	38"A4STO 7FEET 64S.	
GRAVELLY SILT LOAM	7.54	DRY		01	6′		
1	4/2	2. /	1 FSBK	APPROX 30% SUBROUNDED X	_ 7′ _		
				7	_ 8′ _		
				X SOIL SAMPLE	_ 9′ _	-> 2" = FALTORY =	
				ē	10′	PUL FROM	
					_ 11′ _	7 FEET TO =	
		POTTOM	OF EXPL	ORATION.	12′ _	7	
		GROUND	NATER W	MS NOT	13′ _	WEARED IN	
	_				14′ _	GED FABRIC	
					 15′ _	ď	



GHO-TPS

Method of Drill: BACKHOE Boring Diameter: TEST OF WINTED Drilling Co: KEN SMITH	H. HEET				Or Down 1	of I		
Boring Diameter: TEST PIT WINTED Trilling Co: KEN SMITH			Project No.: 8410746.04 Location: STOCKEL PROPERTY Sheet of 1					
Drilling Co: KEN SMITH			Logged By:					
			Total Depth of Hole: 13 Feet 845					
Vegetation: GLMS				5 MARCH 20				
Aspect: 4 2 %				oundwater: Not		OVNTEROS		
Slope: SAMING AT SE	RET AND							
Soil Desc					ے	GHD-MWS		
Description & Remarks	Color (moist)	Moisture	Structure	Comments	Depth	Well Diagram TOC: 21 AGS		
SILT LOAM -	7.51	DRY	MASSIVE		_ 1′ _			
					2′ 2′	7		
					3′ _ 	PLE FROM 21"		
				al	4′ _ 	B4S TO BEET B4S		
				*SAMBE X	5′ _			
					6′ _ 			
	N				7′ _			
GRAVELY SILT LOAM -	7.54		MASSIVE	APPROX 30%	8′ _			
	4/2	DRY	PARTING TO	SUBROUNDED GAFAVEL	_ 9′ _	7		
		·	SINGLE	7	10′ _	2 INCH FACTIVE =		
				* SAMP	_ 11′ _	FRET 645 =		
					12′ _	Vertaining controls to the second of the second to the second of the sec		
	BOTTO	N OF EX	CAVATION.	GROWNDWATEL	_ 13′ -	7		
		NOT E	NCOUNTERE	SHE MIS	14′ _	ENDCAP W/ HOLE WRAPPED IN		
					_ 15′ -	GEO FABRIC:		



GHS-TRO

Project Name: WILDW CREE	K CSD		Project No.:	8410744.04		Sheet of /				
Method of Drill: BACK-HOE			Location: TRINITY VALLEY ELEMENTARY SCH							
Boring Diameter: TOT PIT WIL	DAY . 4 Per	T 845	Logged By: ANN GIONER							
Drilling Co: TIM LA LONDE CONSTRUCTION			Total Depth of Hole: 14 feet 64 8							
Vegetation: 🔾 👫 🕏			Date: /	9 MH 2014			٦			
Aspect: ∠ 2%			Depth to Gro	oundwater: NOT	ENCOUN	MERCA	٦			
Slope: SAMPLES AT 3	FEET AN	D 10 FE	et Bas							
Soil De	scription				4	GHD-MW-V				
Description & Remarks	Color (moist)	Moisture	Structure	Comments	Depth	Well Diagram TOC - 32 AG S				
SAND	251		MASSIVE				1			
		DRY	CONPRETER		_ 1′ _		٦			
	3/3	- 1					7			
				LAYER)	2′		٦			
CEMENTED STAVEL	***************************************	DRY		000		7	٦			
GRAVELLY SAND	251	. 0 1	MASSIVE	APPROX 15%	_ 3′ _		7			
(2.54	DRY	COMPLETED	GRAVEL HOUL ->	_ 3 _	2" BLANK	7			
	3/2				4′	PIL FRAM	7			
SANDY GRAVEL	2.54	DRY>	SINGLE	APPROX 85%		32"AGS TO =				
1	,1	DK	GRAIN	SUBLOUNDED	 - 5' _	4 FORT BGS =				
	3/3	MOIST		GRAVEL W COBBLES	_					
				UPTO 18" IN	 - 6' _					
				DIAMETEL	_ 0 _					
					 _ 7′ _					
~ 1					_ / _					
					_ 8′ _		1			
					_ 8 _	2" -> -	7			
					 9'	PAROLY SLOTTEL -				
	- 1	_		MOISTURE ON GRAVAL	_ 9 _	RICHROM 400				
						14 REET BG &				
		_		*ションーラ	_ 10′ _	n/m/Ships				
				SAMPLE		Contraction of the Contraction o				
					_ 11′ _					
						-				
					_ 12′ _		7			
						- Control of the Cont				
					_ 13′ _		-			
							-			
	B	STTOM OF	EXCAVATION	GROWND WHERE	_ 14′ _	7	+			
	WA	- NOT E	NOINTERES			END CAP WI HOLE	-			
					15′ _	WRAMED IN GEDFARM	21			



GIHSTP7

Project Name: WILLOW CREE	K CSD		Project No.:	8410146.04		Sheet #		
Method of Drill: BACK HOE			Location: TRINITY VALLEY ELEMENTARY SCHOOL					
Boring Diameter: AffRox 4	fact wike		Logged By: ANNA CHOWER					
Drilling Co: TIM LALONDE	CONSTRU	iction !	Total Depth	of Hole: 18 FEET	845			
Vegetation: Calles			Date:	1 MAY 2014				
Aspect: 2 %			Depth to Gro	oundwater: NOT !	ENCOUNT	र्सके		
Slope: SAMILES AT LIFE	ET 645							
	scription					Well		
Description & Remarks	Color (moist)	Moisture	Structure	Comments	Depth	Diagram NO WELL COMPARE		
SAND	2.54	MY	MKSIVE 16MMZICO		_ 1′ _			
SAND W GRAVEL	251		SINGLE	APPOX 15%0	2′			
	2.5Y 3/3	DRY	GRAN	SIBLANNOGS GRAVEL	3′			
GRAVELLY SAND	254	DRY.	SINGLE	APPROX 25%	4′			
	3/3	I		SUBROUNDED GREATEL	5′			
					6′			
		_			7′			
				LOBBLES (UPTO IV"	8′	-		
				APROX & FOOT BGS	9′			
					10′			
					11′ _			
*					12′ _			
				*	13′ _			
					14′ _			
SAND	2.5/ 3/2	DAY TYMAST	GINGLE CAM		15′ _	c		

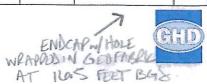
FOTTOM OF EXCAVATION AT APPROXIMATELY 18
FEET BGS. GROWNSWATER WAS NOT ENCOUNTERED.
A WELL WAS NOT COMPLETED IN THIS TEST PIT



GIHD-TPB

Project Name: WILLOW CK	EEK CS	7	Project No.:	8410746.04	!	Sheet I			
Method of Drill: BACK+				FINITY VALLY E					
Boring Diameter: MIROX 4			Logged By: ANNA GOWER						
Drilling Co: TIM LALINDE	CONSTRU	CTION	Total Depth of Hole: 16,5 feet 66,5						
Vegetation: GLASS			Date:	19 MAY 20	-				
Aspect: < 2%			Depth to Gr	oundwater: Not	ENCL	WHERED			
Slope: SAMILES AT 4	FEET, 10 F	EET AND							
	scription		2		<u> </u>	GHD-MW-7			
Description & Remarks	Color (moist)	Moisture	Structure	Comments	Depth	Well Diagram			
SAND	2.51	DRY	MASSIVE COMPARTION	APPLOX-590 SULPOVINDED GRAVEZ	1′				
GRAVELY SAM	2.51	Dey	SINGLE	APROX 2590	 _ 2′ _				
	3/3		GRAN	3/8"->=/4" SVEROVNDED GRAVEL	 3' -				
SANDY GIRAVA	7.54	DRY	SINGLE	-7 V	4′ -	Z'ELANK			
1	2.54	1	GRAN	* SOI AMILE	5′ _	fic from			
	1.			APPROX 60%		2.8' A4510			
				SUBROUNDED GRAVEL	6′ _	U.Sher but			
				<i>i</i> , , , , , , , , , , , , , , , , , , ,	_ 7′_				
					8′ _				
					9′		Seguments of		
				* 501 -> X	10′ _				
				SAMM / V	11′ _	2" FACTORY			
					12′ _	SLOTTED RIC FROM USFLOET	**************************************		
					13′ -	TO 165 FOR			
				SAMPLE AT 16/65.	14′ -				
		MOISTAT		CELOXAT Notings (7.54416)	15' -				
	0 =0	of Exprol	Asimil At II	of Fort Has	L				

GROUND WATER WITS NOT ENCOUNTERED.



Appendix C – Materials Testing Data Sheets

GHD

Sieve Analysis Test -ASTM D422

Willow Creek Community Services District

Project Location: Trinity Valley Elementary School

Job Number: 8410746.04 AP#:

___ Date: Tested By: 6/24/2014 Anna Gower

Sample #: Sample Wt. (dry): GHD-TP1 from 3-3.5 feet bgs 3281.8 grams

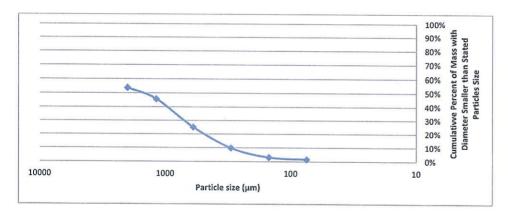
Job Name:

Soil Description:
Sample Location:

Sand- Soil Percolation Suitability Chart Zone 1

Test pit GHD-TP1 from approximately 3 to 3.5 feet bgs

Sieve Size	Particle Size (μm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comments
10	2000	1516.50	46%	1765.30	54%	
16	1180	263.50	54%	1501.80	46%	
30	600	676.40	75%	825.40	25%	
50	300	503.50	90%	321.90	10%	
100	150	221.65	97%	100.25	3%	
200	75	47.30	98%	52.95	2%	
			4			





Sieve Analysis Test -ASTM D422

Willow Creek Community Services District

Job Name: Project Location: Job Number:

AP#:

Mill site

8410746.04

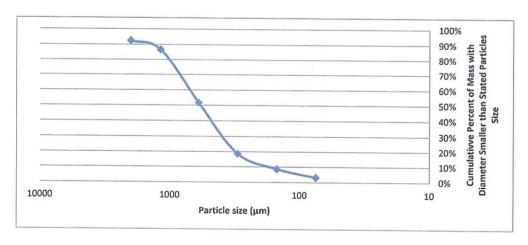
_ Date: Tested By: 6/24/2014 Anna Gower

Sample #: Sample Wt. (dry): GHD-TP1 at 5 feet bgs 1122.2

Soil Description: Sample Location: Sand- Soil Percolation Suitability Chart Zone 1

Test pit GHD-TP1 at approximately 5 feet bgs

Sieve Size	Particle Size (μm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comment
10	2000	84.30	8%	1037.9	92%	
16	1180	65.50	13%	972.4	87%	
30	600	392.30	48%	580.1	52%	
50	300	369.50	81%	210.6	19%	-
100	150	112.67	91%	97.93	9%	
200	75	61.28	97%	36.65	3%	





Job Name: Project Location: Job Number: AP#:

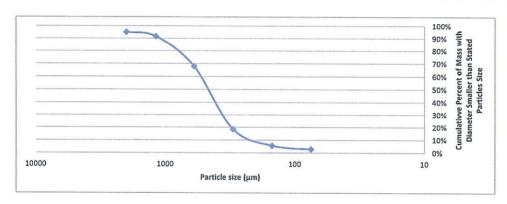
Sieve Analysis Test -ASTM D422

Willow Creek Community Services District

Mill Site			
8410746.04	Date:	6/24/2014	
	Tested By:	Anna Gower	

Sample #: GHD-TP2 from 5 to 5.5 feet bgs Soil Description: Sand- Soil Percolation Suitability Chart Zone 1
Sample Wt. (dry): 1287.0 Sample Location: Test pit GHD-TP2 from approximately 5 to 5.5 feet bgs
grams

Sieve Size	Particle Size (µm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comments
10	2000	60.89	5%	1226.11	95%	Comments
16	1180	45.90	8%	1180.21	92%	
30	600	305.34	32%	874.87	68%	
50	300	634.80	81%	240.07	19%	
100	150	166.53	94%	73.54	6%	
200	75	36.67	97%	36.87	3%	У.



GHD

Sieve Analysis Test -ASTM D422

Job Name: Willow Creek Community Services District

Project Location: Job Number:

AP#:

0440746

Mill site

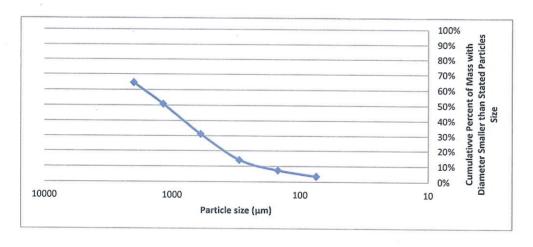
8410746.04

__ Date: Tested By: 6/24/2014 Anna Gower

Sample #: Sample Wt. (dry): GHD-TP3 at 8 feet bgs 1135.7 Soil Description: Sample Location: Sand- Soil Percolation Suitability Chart Zone 1

Test pit GHD-TP3 at approximately 8 feet bgs

	grams	0027			approximately office	
Sieve Size	Particle Size (µm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comment
10	2000	402.00	35%	733.70	65%	
16	1180	156.30	49%	577.40	51%	
30	600	222.97	69%	354.43	31%	
50	300	193.83	86%	160.60	14%	
100	150	76.09	93%	84.51	7%	
200	75	44.30	96%	40.21	4%	
			9			



Sieve Analysis Test -ASTM D422

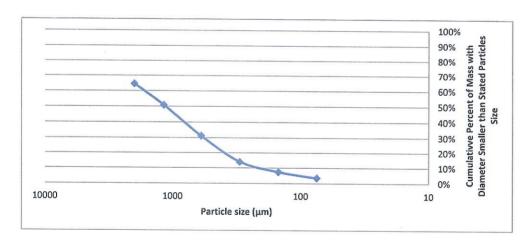
vices District

Job Name:	Willow Creek Community Serv
Project Location:	Mill site
Job Number:	8410746.04

8410746.04 Date: 6/24/2014 AP#: Tested By: Anna Gower

Sample #: GHD-TP3 at 8 feet bgs Soil Description: Sand- Soil Percolation Suitability Chart Zone 1 Sample Wt. (dry): 1135.7 Test pit GHD-TP3 at approximately 8 feet bgs Sample Location: grams

Sieve Size	Particle Size (µm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comments
10	2000	402.00	35%	733.70	65%	comments
16	1180	156.30	49%	577.40	51%	
30	600	222.97	69%	354.43	31%	
50	300	193.83	86%	160.60	14%	
100	150	76.09	93%	84.51	7%	
200	75	44.30	96%	40.21	4%	
			<u>.</u>			



AP#:

Sieve Analysis Test -ASTM D422

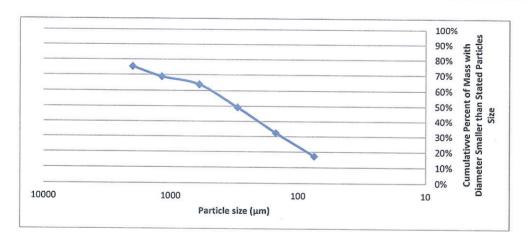
Willow Creek Community Services District

Job Name: Project Location: Stockel property Job Number: 8410746.04

Date: 6/25/2014 Tested By: Anna Gower

Sample #: GHD-TP4 at 7 feet bgs Soil Description: Loamy Sand- Soil Percolation Suitability Chart Zone 2 Sample Wt. (dry): 1203.8 Sample Location: Test pit GHD-TP4 at approximately 7 feet bgs grams

Sieve Size	Particle Size (μm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comments
10	2000	295.6	25%	908.2	75%	
16	1180	78.85	31%	829.35	69%	
30	600	60.58	36%	768.77	64%	
50	300	175.66	51%	593.11	49%	
100	150	203.45	68%	389.66	32%	
200	75	180.96	83%	208.7	17%	



GHD

Sample #:

Sample Wt. (dry):

Sieve Analysis Test -ASTM D422

Job Name: Project Location: Job Number:

AP#:

Willow Creek Community Services District

Stockel property

8410746.04

Date: Tested By: 6/25/2014 Anna Gower

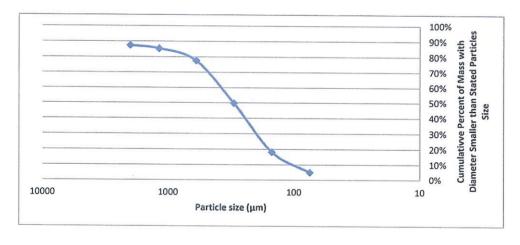
GHD-TP5 at 5 feet bgs Soil D 1311.4 Samp

Soil Description:
Sample Location:

Sand- Soil Percolation Suitability Chart Zone 1

Test pit GHD-TP5 at approximately 5 feet bgs

Charles and the control of the contr	0.50.000.000.000	campic Location.	- i cot p	it only it sut	approximately 5 feet	ugs
	grams					
Sieve Sies	D		Cumulative %	Grams	Cumulative %	
Sieve Size	Particle Size (μm)	Grams Retained	Retained	Passing	Passing	Comment
10	2000	165.33	13%	1146.07	87%	
16	1180	26.5	15%	1119.57	85%	
30	600	101.3	22%	1018.27	78%	
* 50	300	367.4	50%	650.87	50%	
100	150	411.4	82%	239.47	18%	
200	75	173.3	95%	66.17	5%	
						-



GHD

Sieve Analysis Test -ASTM D422

Job Name: Willow Creek Community Services District

Project Location: Trinity Valley Elementary School
Job Number: 8410746.04

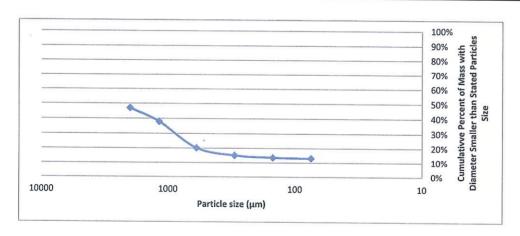
 Job Number:
 8410746.04
 Date:

 AP#:
 Tested By:

6/23/2014 Anna Gower

Sample #: GHD-TP8 at 4 feet bgs Soil Description: Loamy Sand- Soil Percolation Suitability Chart Zone 2
Sample Wt. (dry): 5327.6 Sample Location: Test pit GHD-TP8 at approximately 4 feet bgs
grams

Sieve Size	Particle Size (μm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comments
10	2000	2822.00	53%	2505.60	47%	Comments
16	1180	484.70	62%	2020.90	38%	
30	600	940.30	80%	1080.60	20%	
50	300	272.40	85%	808.20	15%	
100	150	78.50	86%	729.70	14%	
200	75	26.30	87%	703.40	13%	
						-77
		-				



Appendix D – Groundwater Monitoring Well Log

Project: Job No:

Willow Creek Wastewater 12057 / 8410746-04

Site: Groundwater Monitoring Well Log

	_						itoring well	Log				
Well #ID	Week	Date	Time	Well Depth ¹	TOC ² (feet ags)	DTW ³	C=B-A Water Level ⁴ (feet bgs)	Ground Elevation (feet)	Water Elevation (feet)	Rainfall Past 24 hours	Rainfall >0.5" Past Week	Notes and Initials
	1	3/28/2014	1530	13.0	2.5	14.5	12.0	531.6	519.7			
5	2	4/4/2014	1020	13.0	2.5	14.3	11.8	531.6	519.9	0.02	0.3	
¥	3	4/11/2014	745	13.0	2.5	DRY	>13	531.6	UNKOWN	0.0	0.0	
GHD-MW1	4	4/18/2014	740	13.0	2.5	DRY	>13	531.6	UNKOWN			
١	5	4/25/2014	735	13.0	2.5	DRY	>13	531.6	UNKOWN			
	6	5/2/2014	1610	13.0	2.5	DRY	>13	531.6	UNKOWN		90	
	7	5/9/2014	745	13.0	2.5	DRY	>13	531.6	UNKOWN			
	8	5/16/2014	730	13.0	2.5	DRY	>13	531.6	UNKOWN			
		5/23/2014	725	13.0	2.5	DRY	>13	531.6	UNKOWN			
		5/30/2014	730	13.0	2.5	DRY	>13	531.6	UNKOWN			
		6/13/2014	1545	13.0	2.5	DRY	>13	531.6	UNKOWN			
\vdash						5.1.1	- 10	001.0	OTTION			
\vdash	1	3/28/2014	1530	16.5	2.4	15.4	13.0	532.8	519.8			
	2	4/4/2014	1020	16.5	2.4	15.1	12.7	532.8	520.1	0.02		
GHD-MW2	3	4/11/2014	745	16.5	2.4	15.4	13.0	532.8	519.8		0.3	
- N	4	4/18/2014	740	16.5	2.4					0.0	0.0	
[등	5	4/25/2014	735			DRY	>16.5	532.8	UNKOWN	0.0	0.0	
	\vdash			16.5	2.4	DRY	>16.5	532.8	UNKOWN			
\vdash	6	5/2/2014	1610	16.5	2.4	DRY	>16.5	532.8	UNKOWN			
\vdash	7	5/9/2014	745	16.5	2.4	16.5	14.0	532.8	518.7			
\vdash	8	5/16/2014	730	16.5	2.4	DRY	>16.5	532.8	UNKOWN			
_		5/23/2014	725	16.5	2.4	DRY	>16.5	532.8	UNKOWN			
\vdash		5/30/2014	730	16.5	2.4	16.9	14.5	532.8	518.3			
\Box		6/13/2014	1545	16.5	2.4	17.2	14.8	532.8	518.0			
	1	3/28/2014	1530	14.0	1.2	12.3	11.1	530.9	519.8			
62	2	4/4/2014	1020	14.0	1.2	12.0	10.9	530.9	520.0	0.02	0.3	
GHD-MW3	3	4/11/2014	745	14.0	1.2	DRY	>14	530.9	UNKNOWN	0.0	0.0	
₽	4	4/18/2014	740	14.0	1.2	DRY	>14	530.9	UNKNOWN	0.0	0.0	
5	5	4/25/2014	735	14.0	1.2	DRY	>14	530.9	UNKNOWN	0.0	0.0	
	6	5/2/2014	1610	14.0	1.2	DRY	>14	530.9	UNKNOWN			
Н	7	5/9/2014	745	14.0	1.2	13.4	12.2	530.9				
\vdash	8	5/16/2014	730	14.0	1.2				518.7			
\vdash	Ť	5/23/2014	725		1/200	DRY	>14	530.9	UNKNOWN			
H				14.0	1.2	DRY	>14	530.9	UNKNOWN			
⊢	Н	5/30/2014	730	14.0	1.2	14.8	13.6	530.9	517.3			
L		6/13/2014	1345	14.0	1.2	14.1	12.9	530.9	518.0			
_												
	1	3/28/2014	1530	12.0	2.5	DRY	>12	538.8	UNKNOWN			
3	2	4/4/2014	1030	12.0	2.5	DRY	>12	538.8	UNKNOWN	0.02	0.3	
GHD-MW4	3	4/11/2014	745	12.0	2.5	DRY	>12	538.8	UNKNOWN	0.0	0.0	
뭂	4	4/18/2014	740	12.0	2.5	DRY	>12	538.8	UNKNOWN	0.0	0.0	
ľ	5	4/25/2014	735	12.0	2.5	DRY	>12	538.8	UNKNOWN			
	6	5/2/2014	1610	12.0	2.5	DRY	>12	538.8	UNKNOWN			
	7	5/9/2014	745	12.0	2.5	DRY	>12	538.8	UNKNOWN			
	8	5/16/2014	730	12.0	2.5	DRY	>12	538.8	UNKNOWN			
Г		5/23/2014	725	12.0	2.5	DRY	>12	538.8	UNKNOWN			
Г		5/30/2014	730	12.0	2.5	DRY	>12	538.8	UNKNOWN			
		6/15/2014	730	12.0	2.5	DRY	>12	538.8	UNKNOWN			
	1	3/28/2014	1530	13.0	1.8	DRY	>13	533.3	UNKNOWN			
ا ا	2	4/4/2014	1030	13.0	1.8	DRY	>13	533.3	UNKNOWN	0.02	0.3	
W	3	4/11/2014	745	13.0	1.8	DRY	>13	533.3	UNKNOWN	0.02	0.3	
GHD-MW5	4	4/18/2014	740	13.0	1.8	DRY	>13	533.3				
ု ဗ်	5	4/25/2014	735	13.0	1.8				UNKNOWN	0.0	0.0	
	6	5/2/2014	1610	13.0	1.8	DRY	>13	533.3	UNKNOWN			
Н	7	5/9/2014	745	13.0	1.8	DRY	>13	533.3	UNKNOWN		-	
\vdash	8	5/16/2014	730	13.0	1.8		>13	533.3	UNKNOWN			
\vdash	H	5/23/2014	730			DRY	>13	533.3	UNKNOWN			
\vdash	\vdash	100000000000000000000000000000000000000		13.0	1.8	DRY	>13	533.3	UNKNOWN			<u> </u>
\vdash	\vdash	5/30/2014	730	13.0	1.8	DRY	>13	533.3	UNKNOWN			
\vdash	\vdash	6/13/2014	730	13.0	1.8	DRY	>13	533.3	UNKNOWN			
\vdash	Н											4
	1	5/23/2014	725	14.0	2.7	DRY	>14	Not surveyed	UNKNOWN			
9-	2	5/30/2014	730	14.0	2.7	DRY	>14	Not surveyed	UNKNOWN			
¥.	3	6/13/2014	1345	14.0	2.7	DRY	>14	Not surveyed	UNKNOWN			
GHD-MW-6	4											
اقا	5											
	6		1 25 2									
П	1	5/23/2014	725	16.5	2.8	DRY	>16.5	Not surveyed	UNKNOWN			
_	2	5/30/2014	730	16.5	2.8	DRY	>16.5	Not surveyed	UNKNOWN			
GHD-MW-7	3	6/13/2014	1345	16.5	2.8	DRY	>16.5	Not surveyed	UNKNOWN			
0	4			,5.5	2.0	Sixt	- 10.0	. voc surveyed	STATEMONNIN		-	
6	5											
	6											
-							10.00					

Umments:

1. Well dopth = measurement from surface to total dopth below ground surface (togs).

2. TOC = "Top of Casing", measurement of the above ground portion of top of well casing 3. DTW = "Depth To Water", measurement (in feet) from TOC to groundwater surface.

4. Water Lever = DTW - TOC



Sieve Analysis Test -ASTM D422

Job Name:

Willow Creek Community Services District

Project Location: Job Number:

Trinity Valley Elementary School

AP#:

8410746.04

Not applicable

Date:

6/23/2014

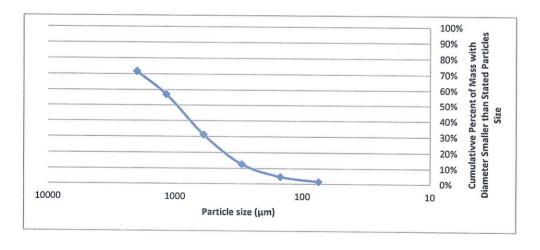
Tested By:

Anna Gower

Sample #: Sample Wt. (dry): GHD-TP7 at 4 feet bgs 3703.8

Soil Description: Sample Location: Sand-Soil Percolation Suitability Chart Zone 1 Test Pit GHD-TP7 at approximately 4 feet bgs

Sieve Size	Particle Size (μm)	Grams Retained	Cumulative % Retained	Grams Passing	Cumulative % Passing	Comments
10	2000	1046.13	28%	2657.67	72%	comments
16	1180	545.60	43%	2112.07	57%	
30	600	958.08	69%	1153.99	31%	
50	300	680.59	87%	473.40	13%	
100	150	300.60	95%	172.80	5%	
200	75	117.30	99%	55.50	1%	
					A	





Project: Job No: Site:

Willow Creek Wastewater 12057 / 8410746-04

Groundwater Monitoring Well Log

	_				Grou	nuwater wor	itoring Well	Log				
					Α	В	C=B-A					-
Well #ID	Week	Date	Time	Well Depth ¹ (feet bgs)	TOC ² (feet ags)	DTW ³ (feet)	Water Level*	Ground Elevation (feet)	Water Elevation (feet)	Rainfall Past 24 hours	Rainfall	Notes and
	1	3/28/2014	1530	13.0	2.5	14.5	12.0	531.6	519.7	Fast 24 IRALS	PU.5 Past Week	Initials
-	2	4/4/2014	1020	13.0	2.5	14.3	11.8	531.6	519.9	0.02	0.3	
GHD-MW1	3	4/11/2014	745	13.0	2.5	DRY	>13	531.6	UNKOWN	0.0	0.0	
[문	4	4/18/2014	740	13.0	2.5	DRY	>13	531.6	UNKOWN			
1	5	4/25/2014	735	13.0	2.5	DRY	>13	531.6	UNKOWN			
	6	5/2/2014	1610	13.0	2.5	DRY	>13	531.6	UNKOWN			
L	7	5/9/2014	745	13.0	2.5	DRY	>13	531.6	UNKOWN			
	8	5/16/2014	730	13.0	2.5	DRY	>13	531.6	UNKOWN			
		5/23/2014	725	13.0	2.5	DRY	>13	531.6	UNKOWN			
		5/30/2014	730	13.0	2.5	DRY	>13	531.6	UNKOWN			
		6/13/2014	1545	13.0	2.5	DRY	>13	531.6	UNKOWN			
	1	3/28/2014	1530	16.5	2.4	15.4	13.0	532.8	519.8			
2	2	4/4/2014	1020	16.5	2.4	15.1	12.7	532.8	520.1	0.02	0.3	
GHD-MW2	3	4/11/2014	745	16.5	2.4	15.4	13.0	532.8	519.8	0.0	0.0	
호	4	4/18/2014	740	16.5	2.4	DRY	>16.5	532.8	UNKOWN	0.0	0.0	
٥	5	4/25/2014	735	16.5	2.4	DRY	>16.5	532.8	UNKOWN	0.0	0.0	
	6	5/2/2014	1610	16.5	2.4	DRY	>16.5	532.8	UNKOWN			
	7	5/9/2014	745	16.5	2.4	16.5	14.0	532.8	518.7			
	8	5/16/2014	730	16.5	2.4	DRY	>16.5	532.8	UNKOWN			
		5/23/2014	725	16.5	2.4	DRY	>16.5	532.8	UNKOWN			
Г		5/30/2014	730	16.5	2.4	16.9	14.5	532.8	518.3			
Г		6/13/2014	1545	16.5	2.4	17.2	14.8	532.8	518.0			
							. 1.0	552.0	310.0			
	1	3/28/2014	1530	14.0	1.2	12.3	11.1	530.9	519.8			
_	2	4/4/2014	1020	14.0	1.2	12.0	10.9	530.9	519.8	0.02	0.3	
GHD-MW3	3	4/11/2014	745	14.0	1.2	DRY	>14	530.9	UNKNOWN	0.02		
후	4	4/18/2014	740	14.0	1.2	DRY	>14	530.9	UNKNOWN	0.0	0.0	
ច	5	4/25/2014	735	14.0	1.2	DRY	>14	530.9	UNKNOWN	0.0	0.0	
П	6	5/2/2014	1610	14.0	1.2	DRY	>14	530.9	UNKNOWN			
	7	5/9/2014	745	14.0	1.2	13.4	12.2	530.9	518.7			
	8	5/16/2014	730	14.0	1.2	DRY	>14	530.9	UNKNOWN			
		5/23/2014	725	14.0	1.2	DRY	>14	530.9				
П	\vdash	5/30/2014	730	14.0	1.2	14.8	13.6	530.9	UNKNOWN			
П		6/13/2014	1345	14.0	1.2	14.1	12.9		517.3			
Н		511012011	10.0	14.0	1.2	14.1	12.9	530.9	518.0			
	1	3/28/2014	1530	12.0	2.5	DRY	>40	500.0	I DUGUELLO I			
	2	4/4/2014	1030	12.0	2.5	DRY	>12	538.8 538.8	UNKNOWN			
GHD-MW4	3	4/11/2014	745	12.0	2.5	DRY	>12	538.8	UNKNOWN	0.02	0.3	
글	4	4/18/2014	740	12.0	2.5	DRY	>12	538.8	UNKNOWN	0.0	0.0	
اقا	5	4/25/2014	735	12.0	2.5	DRY	>12	538.8	UNKNOWN	0.0	0.0	
	6	5/2/2014	1610	12.0	2.5	DRY	>12	538.8	UNKNOWN			
	7	5/9/2014	745	12.0	2.5	DRY	>12	538.8	UNKNOWN			
	8	5/16/2014	730	12.0	2.5	DRY	>12	538.8	UNKNOWN			
П		5/23/2014	725	12.0	2.5	DRY	>12	538.8	UNKNOWN			
		5/30/2014	730	12.0	2.5	DRY	>12	538.8	UNKNOWN			
П		6/15/2014	730	12.0	2.5	DRY	>12	538.8	UNKNOWN			
П							- 16	550.0	SIAICIAOAAIA			
\Box	1	3/28/2014	1530	13.0	1.8	DRY	>13	533.3	UNKNOWN			
[,	2	4/4/2014	1030	13.0	1.8	DRY	>13	533.3	UNKNOWN	0.02		
GHD-MW5	3	4/11/2014	745	13.0	1.8	DRY	>13	533.3	UNKNOWN	0.02	0.3	
를	4	4/18/2014	740	13.0	1.8	DRY	>13	533.3	UNKNOWN	0.0	0.0	
ō	5	4/25/2014	735	13.0	1.8	DRY	>13	533.3	UNKNOWN	0.0	0.0	
	6	5/2/2014	1610	13.0	1.8	DRY	>13	533.3	UNKNOWN			
	7	5/9/2014	745	13.0	1.8	DRY	>13	533.3	UNKNOWN			
	8	5/16/2014	730	13.0	1.8	DRY	>13	533.3	UNKNOWN			
		5/23/2014	725	13.0	1.8	DRY	>13	533.3	UNKNOWN			
П		5/30/2014	730	13.0	1.8	DRY	>13	533.3	UNKNOWN			
		6/13/2014	730	13.0	1.8	DRY	>13	533.3	UNKNOWN			
П							. ,0	000.0	CHANGANIA			
\Box	1	5/23/2014	725	14.0	2.7	DRY	>14	Not surveyed	UNKNOWN			
ا ۾	2	5/30/2014	730	14.0	2.7	DRY	>14	Not surveyed	UNKNOWN			
× I	3	6/13/2014	1345	14.0	2.7	DRY	>14	Not surveyed	UNKNOWN			
GHD-MW-6	4							ou voyou	SINGOVIN			
0	5											
	6											
\vdash	1	5/23/2014	725	16.5	2.8	DRY	>16.5	Not surveyed	UNKNOWN			
1,1	2	5/30/2014	730	16.5	2.8	DRY	>16.5		UNKNOWN			
GHD-MW-7	3	6/13/2014	1345	16.5	2.8	DRY	>16.5	Not surveyed Not surveyed	UNKNOWN			
10	4							ou veyed	SININIOVVIA			
ō	5								·			
l t	6											
	_											

Comments:

1. Well depth = measurement from surface to total depth below ground surface (bgs).

2. TOC = "Top of Casing", measurement of the above ground portion of top of well casing

3. DTW = "Depth To Water", measurement (in feet) from TOC to groundwater surface.

4. Water Lever = DTW - TOC

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